



R & M STEEL COMPANY

P.O. Box 580
Caldwell, Idaho 83606

STEEL BUILDING SYSTEMS

Phone: 1-208-454-1800 Toll Free: 1-866-454-1800
Fax: 1-208-454-1801 Toll Free: 1-866-454-1801

DRAWING APPROVAL

DATE: 2/15/2024

PHONE: 208-602-6106

TO FAX NUMBER:

TO: **CAMP PINEWOOD**

ATTENTION: **KYLE MASCHEK**

NUMBER OF PAGES TRANSMITTED INCLUDING THIS COVER SHEET: **15**

REPLY REQUESTED: _____ URGENT **XXX** ASAP _____ NONE NEEDED

REGARDING JOB: **CAMP PINEWOOD
MCCALL, ID**

QUOTE #: **EG-231219-5 DS REV.**

APPROVAL REVISION #: **1**

**Please Review the Attached Drawings
Check the Appropriate Comment Box
Sign Your Approval in the Box Below
Email to: projectmanagement@rmsteel.com**

| | |
|---|------------------------------------|
| Documents For Approval Not To Be Used For Construction | |
| <input type="checkbox"/> | Approved With No Changes |
| <input type="checkbox"/> | Approved With Changes |
| <input type="checkbox"/> | Resubmit For Approval with Changes |
| <p>These documents set forth R & M Steel's interpretation of the building requirements as represented by your Acceptance of Quotation. Changes must be marked clearly in red ink. Any alteration or deviation from the Acceptance of Quotation will be subject to a written Change Order agreed-to and executed by both parties, and same will become a charge over and above the agreed-to Purchase Price.</p> | |
| _____ | _____ |
| Authorized Buyer's Signature | Date |

FROM: **PROJECT MANAGEMENT**

Rev. #3 Revised 2/22/2016

www.rmsteel.com

www.aviationbuildingsystem.com

Steel Building Excellence • Since 1969

CUSTOMER INFORMATION

CUSTOMER NAME: CAMP PINWOOD
 ADDRESS: 300 N MISSION ST.
 MCCALL, ID 83638

PROJECT NAME: CAMP PINWOOD
 PROJECT LOCATION: MCCALL, ID

GENERAL NOTES

1. MATERIALS

| | | |
|--------------------------------|-------------------------|-----------|
| | ASTM DESIGNATION | |
| STRUCTURAL STEEL PLATE | A529 OR A572 OR A1011SS | GRADE 55 |
| FLANGE MATERIAL | A529 | GRADE 55 |
| COLD FORMED LIGHT GAUGE SHAPES | A1011SS | GRADE 55 |
| STRUCTURAL CABLES | A475 | GRADE EHS |
| HOT ROLLED MILL SHAPE | A992 | GRADE 50 |
| HOLLOW STRUCTURAL SECTIONS | A500 | GRADE B |
| PBR36 ROOF AND WALL PANELS | A653 OR A792 | GRADE 80 |
| STANDING SEAM ROOF | A653 OR A792 | GRADE 50 |
| BOLTS | A325 | A325 |
| BOLTS | GRADE 5 | GRADE 5 |
2. DESIGN
 - A. ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBER ARE DESIGNED IN ACCORDANCE WITH THE AISC 360-16 "SPECIFICATIONS FOR THE DESIGN, FABRICATING AND ERECTION OF STRUCTURAL STEEL BUILDING", ALLOWABLE STRESS DESIGN.
 - B. ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH AISI S100-16 "SPECIFICATIONS FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS".
 - C. ALL WELDING OF STRUCTURAL STEEL IS BASED ON AWS D1.1 2017 "STRUCTURAL WELDING CODE".
3. HIGH STRENGTH BOLT CONNECTIONS:

ALL HIGH STRENGTH BOLTS ARE TYPE ASTM A325 AND ARE TO BE INSTALLED ACCORDING TO THE "SNUG-TIGHT" CONDITIONS AS DEFINED BY THE, RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, UNLESS NOTED OTHERWISE.

ALSO, NOTE THAT BOLTS IN STANDARD HOLES DO NOT REQUIRE WASHERS PER THE, RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, SECTION 6 (REFERENCE STEEL CONSTRUCTION AISC MANUAL 360-16)
4. A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTED CONNECTIONS ARE SUBJECT TO AXIAL TENSION AND OR SLIP CRITICAL. AS SUCH THE BOLTS MUST BE FULLY PRE-TENSIONED AND INSPECTED IN ACCORDANCE WITH THE AISC 360-16 SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS AND THE APPLICABLE BUILDING CODE. WASHERS ARE NOT REQUIRED WHEN THE "TURN OF THE NUT" TIGHTENING PROCEDURE IS USED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE PROPER BOLT TIGHTNESS.
5. STRUCTURAL PRIMER

ALL STRUCTURAL MEMBERS WILL BE GIVEN ONE COAT OF MANUFACTURER'S STANDARD RUST-INHIBITIVE PRIMER MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-6645. THIS IS NOT A FINISH COAT AND IS NOT INTENDED FOR PROLONGED EXPOSURE TO THE ELEMENTS. REFERENCE AISC 360-16, CODE OF STANDARD PRACTICE SECTION 6.5.1-6.5.4.

6. BUILDER / CONTRACTOR RESPONSIBILITIES

R & M STEEL COMPANY STANDARD PRODUCT SPECIFICATIONS APPLY AND R & M STEEL COMPANY DESIGN, FABRICATION, QUALITY CONTROL STANDARDS AND TOLERANCE WILL GOVERN. IN CASE OF DISCREPANCIES BETWEEN R & M STEEL COMPANY'S PLANS AND PLANS FOR OTHER TRADES R & M STEEL PLANS SHALL GOVERN. (SECTION 3.3 AISC 303-16 CODE OF STANDARD PRACTICES.)

IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE OR FEDERAL AGENCIES AS REQUIRED.

APPROVAL OF R & M STEEL COMPANY'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF R & M STEEL COMPANY'S INTERPRETATION OF THE PURCHASE ORDER. (SECTION 4.2.1 AISC 303.16 CODE OF STANDARD PRACTICES.)

THE BUILDER / CONTRACTOR OR A/E FIRM IS RESPONSIBLE FOR THE OVERALL PROJECT. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIAL NOT FURNISHED BY R & M STEEL COMPANY ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER. R & M STEEL COMPANY ASSUMPTIONS WILL GOVERN.

THE BUILDER / CONTRACTORS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH R & M STEEL COMPANY'S BUILDING "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH AND INSTALL. (SECTION 7.9.1 AISC 303.16 CODE OF STANDARD PRACTICES.)

THE DESIGN OF THE ANCHOR BOLT EMBEDMENT LENGTH IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER. THE LENGTH PROVIDED BY R & M STEEL IS AN ESTIMATED LENGTH AND SHOULD BE ADJUSTED ACCORDING TO THE FOUNDATION DESIGN.

BUILDING LOADS / DESCRIPTION:

WIDTH: 60 LENGTH: 130 HEIGHT: 22 / 22

ROOF PITCH: 4.0:12 / 4.0:12

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 18

THE CONTRACTOR / BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

| | | |
|------------------------------|--------|-----------------------------|
| ROOF DEAD LOAD: | 3.48 | PSF (ROOF PANELS & PURLINS) |
| COLLATERAL LOAD: | 5.00 | PSF |
| ROOF LIVE LOAD: | 20.00 | PSF |
| MIN. UNIFORM ROOF SNOW LOAD: | 120.00 | PSF |
| GROUND SNOW LOAD: (Pg) | 150.00 | PSF |
| ROOF SNOW LOAD: (Pf) | 105.00 | PSF |
| BASIC WIND SPEED: | 115 | MPH |
| SEISMIC COEFFICIENT: | 0.632 | |

IMPORTANCE FACTORS:

| | |
|------------|------|
| WIND LOAD: | 1.00 |
| SNOW LOAD: | 1.00 |
| SEISMIC: | 1.00 |

ROOF PANELS:

COLOR: ROOF COLOR

WALL PANELS:

COLOR: WALL COLOR

TRIM COLORS:

| | |
|------------------|------------|
| GABLE: | TRIM COLOR |
| EAVE: | TRIM COLOR |
| CORNER: | TRIM COLOR |
| DOOR & WINDOW: | TRIM COLOR |
| GUTTER: | -- |
| DOWNSPOUTS: | -- |
| BASE (OPTIONAL): | WALL COLOR |
| 8" JAMB/HEAD: | TRIM COLOR |

SOFFIT PANEL:

| | |
|------------|--|
| GABLE EXT: | |
| EAVE EXT: | |
| CANOPY: | |

LINER PANEL:

| | |
|--------|--|
| LEFT : | |
| RIGHT: | |
| FRONT: | |
| BACK : | |
| ROOF : | |

Pf = 0.7 Ce Ct I Pg
 Ce = 1
 Ct = 1.0
 I = 1
 Pg = 150.0 PSF
 Pf = 105.00 PSF
 UNBALANCED LOAD = 160.4 PSF

EARTHQUAKE DESIGN DATA:

INPUT:
 Occupancy Category: II - Normal
 Seismic Importance Factor: 1.00
 Mapped Response (Short), Ss: 0.44
 Mapped Response (1 sec.), S1: 0.14
 Site Class: d - DEFAULT

RESULT:
 Seismic Design Category, SDC: D
 Basic Seismic-Force-Resisting Systems: OCBF, OMF
 Analysis Procedure Used: Equivalent Lateral

FORCE:
 Site Coeff (Short), Fa: 1.4520
 Site Coeff (1 sec.), Fv: 2.3180
 Max. Design Response (Short), Sms: 0.63
 Max. Design Response (1 sec.), Sml: 0.3268
 Design Response (Short), Sds: 0.42
 Design Response (1 sec.), Sd1: 0.22
 Approx. Period (Moment), Ta: 0.3911
 Approx. Period (Brace), Tb: 0.2369
 Rigid Frame Deflection Limit (Seis): 65
 Wind Bent Deflection Limit (Seis): 65

DES_CALC:
 Seismic Forces:

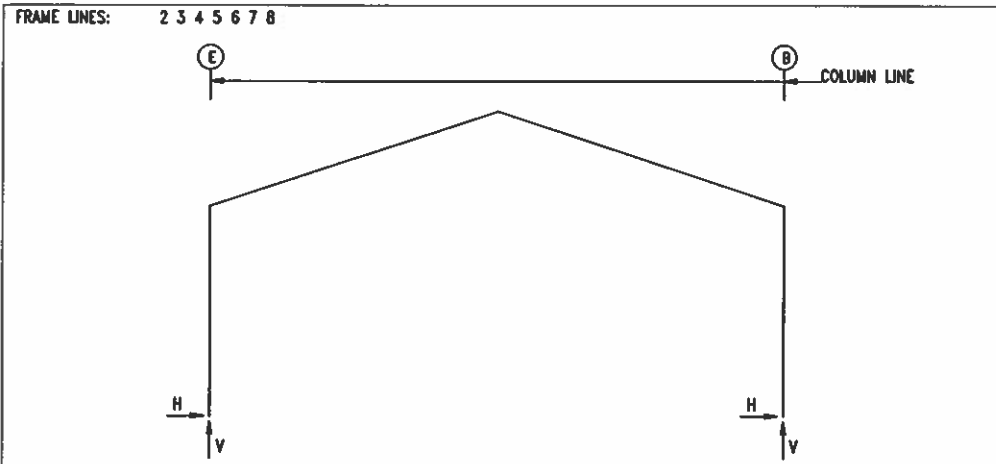
| | | |
|---------------|------------------|--------------------|
| Roof Bracing: | Endwall Bracing: | End Plates, Frame: |
| R = 3.2500 | R = 3.2500 | R = 3.2500 |
| Rho = 1.3000 | Omega = 2.0000 | Omega = 3.0000 |
| Cs = 0.13 | Cs = 0.13 | |

| | | |
|-------------------|---------------|--------------|
| Sidewall Bracing: | Rigid Frames: | Wind Bent |
| R = 3.2500 | R = 3.25 | R = 3.2500 |
| Omega = 2.0000 | Rho = 1.3000 | Rho = 1.3000 |
| Cs = 0.13 | Cs = 0.13 | Cs = 0.13 |

Total Base Shear:
 Longitudinal Force, V = 54.58 (k)
 Transverse Force, V = 33.28 (k)



| | | |
|--|--------------|----------------|
| R & M STEEL COMPANY P.O. Box 580 Caldwell, Idaho 83606 208-454-1800 Fax 208-454-1801 | | |
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINWOOD | | DRAWN BY |
| CAMP PINWOOD | | SB |
| | | DRAWING NUMBER |
| | | OF |



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES
THE VERTICAL COMPONENT OF BRACING REACTION IS INCLUDED IN VERTICAL REACTIONS.

| Frame Line | Column Line | Load Id | Hmax | Vmax | Load Id | Hmin | Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Grout (in) |
|------------|-------------|---------|-------|------|---------|-------|------|--------------|-------|----------------------|--------|-------|------------|
| 2* | E | 5 | 26.4 | 66.3 | 2 | -3.6 | -1.9 | 4 | 1.000 | 8.000 | 11.00 | 0.750 | 0.0 |
| | | 1 | 22.0 | 88.7 | 4 | 0.6 | -7.6 | | | | | | |
| 2* | B | 3 | 3.6 | -1.9 | 5 | -26.4 | 66.3 | 4 | 1.000 | 8.000 | 11.00 | 0.750 | 0.0 |
| | | 1 | -22.0 | 88.7 | 4 | -0.6 | -7.6 | | | | | | |

2* Frame lines: 2 3 4 5 6 7 8

RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column | Dead | Collateral | Live | Snow | Wind_Left1 | Wind_Right1 | Wind_Left2 | Wind_Right2 | | | | |
|------------|--------|------|------------|------|------|------------|-------------|------------|-------------|------|-------|-----|-------|
| 2* | E | 1.2 | 4.9 | 1.1 | 2.8 | 3.8 | 14.5 | 19.8 | 81.0 | -5.8 | -16.1 | 2.6 | -9.8 |
| 2* | B | -1.2 | 4.9 | -1.1 | 2.8 | -3.8 | 14.5 | -19.8 | 81.0 | -2.6 | -9.8 | 5.8 | -16.1 |

| Frame Line | Column | Wind_Left2 | Wind_Right2 | Wind_Long1 | Wind_Long2 | Seismic_Left | Seismic_Right | | | | | | |
|------------|--------|------------|-------------|------------|------------|--------------|---------------|------|-------|------|------|-----|------|
| 2* | E | -7.1 | -8.1 | 1.2 | -1.9 | 1.7 | -16.5 | 0.5 | -15.7 | -5.2 | -4.3 | 5.2 | 3.3 |
| 2* | B | -1.2 | -1.9 | 7.1 | -8.1 | -0.5 | -15.7 | -1.7 | -16.5 | -5.2 | 4.3 | 5.2 | -3.3 |

| Frame Line | Column | Seismic_Long | MIN_SNOW | E1UNB_SL_L | E1UNB_SL_R | | | | |
|------------|--------|--------------|----------|------------|------------|-------|------|-------|------|
| 2* | E | 0.0 | -14.7 | 24.2 | 58.5 | 20.7 | 59.8 | 20.7 | 33.0 |
| 2* | B | 0.0 | -14.7 | -24.2 | 58.5 | -20.7 | 33.0 | -20.7 | 59.8 |

2* Frame lines: 2 3 4 5 6 7 8

BUILDING BRACING REACTIONS

| Wall Loc | Col Line | ± Reactions(k) | Panel Shear (lb/ft) | | | |
|----------|----------|----------------|---------------------|------|------|--|
| | | Wind | Seismic | Wind | Seis | |
| L_SW | 1 | 1.6 | 2.2 | 3.1 | 4.3 | |
| | 3,4 | 2.8 | 3.5 | 11.8 | 14.7 | |
| | 5,6 | 2.8 | 3.5 | 11.8 | 14.7 | |
| | 7,8 | 2.8 | 3.5 | 11.8 | 14.7 | |
| R_SW | 9 | 1.6 | 2.1 | 3.1 | 4.1 | |
| | C,D | 2.8 | 3.5 | 11.8 | 14.7 | |
| | 6,5 | 2.8 | 3.5 | 11.8 | 14.7 | |
| | 4,3 | 2.8 | 3.5 | 11.8 | 14.7 | |

Reactions for seismic represent shear force, Eh

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

| Frame Line | Col Line | Dead | Collat | Live | Snow | Wind_Left1 | Wind_Right1 | Wind_Left2 | Wind_Right2 | Wind Press |
|------------|----------|------|--------|------|------|------------|-------------|------------|-------------|------------|
| 1 | E | 1.3 | 0.5 | 4.0 | 23.5 | -1.6 | -6.2 | 0.0 | -0.4 | -2.3 |
| 1 | D | 1.5 | 1.0 | 3.7 | 19.5 | 0.0 | -3.8 | 1.6 | -6.6 | -5.7 |
| 1 | C | 1.5 | 1.0 | 3.7 | 19.5 | 0.0 | -3.8 | 0.0 | -6.1 | -5.7 |
| 1 | B | 1.3 | 0.5 | 4.0 | 23.5 | 0.0 | -3.2 | 0.0 | -3.9 | -2.3 |

| Frame Line | Col Line | Wind Suct | Wind_Long1 | Wind_Long2 | Seis_Left | Seis_Right | Seis Long | MIN_SNOW | | | | |
|------------|----------|-----------|------------|------------|-----------|------------|-----------|----------|-----|------|-----|------|
| 1 | E | 2.6 | 0.0 | -1.0 | -1.0 | -3.4 | -3.1 | -4.2 | 0.0 | 5.2 | 0.0 | 8.4 |
| 1 | D | 6.4 | 1.0 | -6.2 | 0.0 | -0.9 | 0.0 | 4.2 | 3.1 | -5.2 | 0.1 | 22.8 |
| 1 | C | 6.4 | 0.0 | -2.4 | 0.0 | -4.3 | 0.0 | 0.3 | 0.0 | -0.3 | 0.1 | 22.8 |
| 1 | B | 2.6 | 0.0 | -1.9 | 0.0 | -2.9 | 0.0 | -0.2 | 0.0 | 0.2 | 0.0 | 8.4 |

| Frame Line | Col Line | E1UNB_SL_L | E1UNB_SL_R | | |
|------------|----------|------------|------------|-----|------|
| 1 | E | 0.0 | 8.3 | 0.0 | 2.0 |
| 1 | D | 0.0 | 31.4 | 0.0 | 7.7 |
| 1 | C | 0.0 | 7.8 | 0.0 | 31.3 |
| 1 | B | 0.0 | 2.0 | 0.0 | 8.3 |

| Frame Line | Col Line | Dead | Collat | Live | Snow | Wind_Left1 | Wind_Right1 | Wind_Left2 | Wind_Right2 | Wind Press |
|------------|----------|------|--------|------|------|------------|-------------|------------|-------------|------------|
| 9 | B | 1.3 | 0.5 | 4.0 | 24.0 | 0.0 | -4.9 | 0.0 | -6.0 | -1.6 |
| 9 | C | 1.5 | 1.0 | 3.7 | 19.1 | -1.6 | -6.3 | 0.0 | 0.2 | -5.7 |
| 9 | D | 1.5 | 1.0 | 3.7 | 19.1 | 0.0 | 0.2 | 1.6 | -6.3 | -5.7 |
| 9 | E | 1.3 | 0.5 | 4.0 | 24.0 | 0.0 | -6.0 | 0.0 | -4.9 | -2.3 |

| Frame Line | Col Line | Wind Suct | Wind_Long1 | Wind_Long2 | Seis_Left | Seis_Right | Seis Long | MIN_SNOW | | | | |
|------------|----------|-----------|------------|------------|-----------|------------|-----------|----------|-----|------|-----|------|
| 9 | B | 2.6 | 0.0 | -2.8 | 0.0 | -1.9 | 0.0 | 0.2 | 0.0 | -0.1 | 0.0 | 8.4 |
| 9 | C | 6.4 | 0.0 | -3.0 | -1.0 | -3.8 | -3.1 | -4.4 | 0.0 | 4.2 | 0.1 | 22.8 |
| 9 | D | 6.4 | 1.0 | -3.8 | 0.0 | -3.0 | 0.0 | 4.2 | 3.1 | -4.4 | 0.1 | 22.8 |
| 9 | E | 2.6 | 0.0 | -1.9 | 0.0 | -2.8 | 0.0 | -0.1 | 0.0 | 0.2 | 0.0 | 8.4 |

| Frame Line | Col Line | E2UNB_SL_L | E2UNB_SL_R | | |
|------------|----------|------------|------------|-----|------|
| 9 | B | 0.0 | 8.3 | 0.0 | 2.0 |
| 9 | C | 0.0 | 31.3 | 0.0 | 7.8 |
| 9 | D | 0.0 | 7.8 | 0.0 | 31.3 |
| 9 | E | 0.0 | 2.0 | 0.0 | 8.3 |

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frame Line | Col Line | Load Id | Hmax | Vmax | Load Id | Hmin | Vmin | Bolt(in) Qty | Dia | Base_Plate(in) Width | Length | Thick | Grout (in) |
|------------|----------|---------|------|------|---------|------|------|--------------|-------|----------------------|--------|-------|------------|
| 1 | E | 6 | 1.6 | -3.0 | 7 | -1.4 | -1.2 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 1 | 0.0 | 25.4 | 6 | 1.6 | -3.0 | | | | | | |
| 1 | D | 8 | 3.8 | -3.1 | 9 | -3.4 | -2.8 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 10 | 0.0 | 33.9 | 8 | 3.8 | -3.1 | | | | | | |
| 1 | C | 8 | 3.8 | -2.7 | 7 | -3.4 | -1.7 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 11 | 0.0 | 33.8 | 8 | 3.8 | -2.7 | | | | | | |
| 1 | B | 8 | 1.6 | -1.6 | 7 | -1.4 | -0.9 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 1 | 0.0 | 25.4 | 8 | 1.6 | -1.6 | | | | | | |
| 9 | B | 8 | 1.6 | -2.8 | 9 | -1.4 | -0.9 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 1 | 0.0 | 25.8 | 8 | 1.6 | -2.8 | | | | | | |
| 9 | C | 12 | 3.8 | -3.0 | 7 | -3.4 | -1.4 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 13 | 0.0 | 33.8 | 12 | 3.8 | -3.0 | | | | | | |
| 9 | D | 14 | 3.8 | -3.0 | 9 | -3.4 | -1.4 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 15 | 0.0 | 33.8 | 14 | 3.8 | -3.0 | | | | | | |
| 9 | E | 6 | 1.6 | -2.8 | 7 | -1.4 | -0.9 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 1 | 0.0 | 25.8 | 6 | 1.6 | -2.8 | | | | | | |

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 60.0
 - Length (ft) = 130.0
 - Eave Height (ft) = 22.0 / 22.0
 - Roof Slope (rise/12) = 4.0 / 4.0
 - Dead Load (psf) = 3.5
 - Collateral Load (psf) = 5.0
 - Live Load (psf) = 20.0
 - Min. Uniform Roof Snow Load (psf) = 120.0
 - Ground Snow Load (psf) (Pg) = 150.0
 - Roof Snow Load (psf) (Pi) = 105.0
 - Wind Speed (mph) = 115.0
 - Wind Code = IBC 18
 - Exposure = C
 - Closure = Enclosed
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = D
 - Seismic Coeff (Fa/Sa) = 0.63
- Loading conditions are:
 - 1 Dead+Collateral+Snow+Slide_Snow
 - 2 0.6Dead+0.6Wind_Left2
 - 3 0.6Dead+0.6Wind_Right2
 - 4 0.54Dead+0.7Seismic_LongL
 - 5 Dead+Collateral+MIN_SNOW
 - 6 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
 - 7 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 - 8 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
 - 9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
 - 10 Dead+Collateral+E1UNB_SL_L
 - 11 Dead+Collateral+E1UNB_SL_R
 - 12 0.6Dead+0.6Wind_Left2+0.6Wind_Suction
 - 13 Dead+Collateral+E2UNB_SL_L
 - 14 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
 - 15 Dead+Collateral+E2UNB_SL_R

Pf = 0.7 Ce Ct | Pg
Ce = 1
Ct = 1.0
I = 1
Pg = 150.0 PSF
Pi = 105.00 PSF
UNBALANCED LOAD = 160.4 PSF

ANCHOR BOLT SUMMARY

| Qty | Locate | Dia (in) | Type | Proj (in) |
|-----|---------|----------|------|-----------|
| 4 | Jamb | 1/2" | GR36 | 1.00 |
| 32 | Endwall | 3/4" | GR36 | 2.00 |
| 56 | Frame | 1" | GR36 | 2.00 |

NOTE: DESIGNED FOR FUTURE 30'x130'x22' HSSS PARTIALLY ENCLOSED LEAN-TO @ GRID 'B' & 'E'

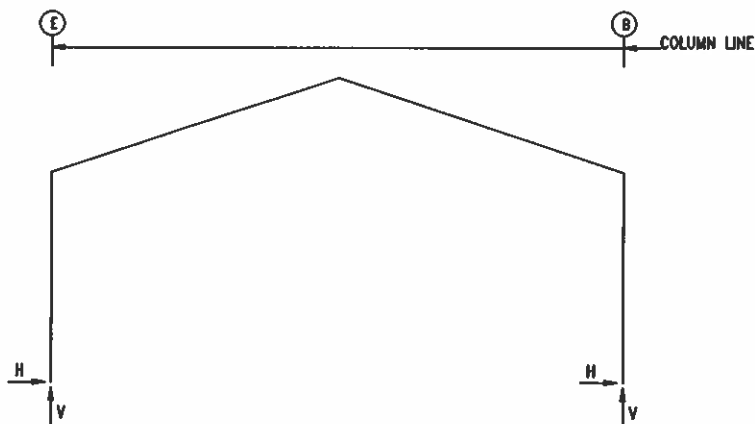
REACTIONS W/ (2) LEAN TO'S ATTACHED

R & M STEEL COMPANY
P.O. Box 580
Caldwell, Idaho 83606
208-454-1800 Fax 208-454-1801

SCALE: _____ JOB LOCATION: _____ REVISION: _____
DATE: 2/14/24 MCCALL, ID

CAMP PINWOOD DRAWN BY: SB
CAMP PINWOOD DRAWING NUMBER: _____ OF: _____

FRAME LINES: 2 3 4 5 6 7 8



ENDWALL COLUMN:

BASIC COLUMN REACTIONS (k)

| Frm Line | Col Line | Dead Vert | Collat Vert | Live Vert | ---Snow--- | | Wind_Left1 | | Wind_Right1 | | Wind_Left2 | | Wind_Right2 | |
|----------|----------|-----------|-------------|-----------|------------|------|------------|------|-------------|-------|------------|------|-------------|------|
| | | | | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert |
| 1 | E | 0.6 | 0.4 | 1.3 | -0.2 | 6.9 | 0.0 | -0.5 | 0.0 | 5.9 | -2.6 | -4.5 | 0.0 | 0.9 |
| 1 | D | 1.5 | 1.0 | 3.9 | 0.0 | 20.5 | 0.8 | -6.8 | 4.6 | -11.0 | 0.0 | -0.5 | 1.2 | -3.8 |
| 1 | C | 1.5 | 1.0 | 3.7 | 0.0 | 19.2 | 0.0 | -3.7 | 0.0 | -5.9 | 0.0 | -1.5 | 0.0 | -3.8 |
| 1 | B | 1.4 | 0.5 | 4.0 | 0.0 | 24.0 | 0.0 | -3.3 | 0.0 | -4.1 | 0.0 | -1.2 | 0.0 | -2.0 |

| Frm Line | Col Line | Wind Press Horz | Wind Suct Horz | Wind_Long1 | | Wind_Long2 | | Seis_Left | | Seis_Right | | Seis Long | -MIN_SNOW-- | |
|----------|----------|-----------------|----------------|------------|------|------------|------|-----------|------|------------|------|-----------|-------------|------|
| | | | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | | | |
| 1 | E | -2.3 | 2.6 | 0.0 | -1.0 | -1.0 | -3.4 | -3.1 | -4.2 | 0.0 | 5.2 | 0.0 | 0.0 | 8.4 |
| 1 | D | -5.7 | 6.4 | 1.0 | -6.2 | 0.0 | -0.9 | 0.0 | 4.2 | 3.1 | -5.2 | 0.1 | 0.0 | 22.8 |
| 1 | C | -5.7 | 6.4 | 0.0 | -2.4 | 0.0 | -4.3 | 0.0 | 0.3 | 0.0 | -0.3 | 0.1 | 0.0 | 22.8 |
| 1 | B | -2.3 | 2.6 | 0.0 | -1.9 | 0.0 | -2.9 | 0.0 | -0.2 | 0.0 | 0.2 | 0.0 | 0.0 | 8.4 |

| Frm Line | Col Line | E1UNB_SL_L- | | E1UNB_SL_R- | |
|----------|----------|-------------|------|-------------|------|
| | | Horz | Vert | Horz | Vert |
| 1 | E | 0.0 | 8.3 | 0.0 | 2.0 |
| 1 | D | 0.0 | 31.4 | 0.0 | 7.8 |
| 1 | C | 0.0 | 7.8 | 0.0 | 25.8 |
| 1 | B | 0.0 | 2.0 | 0.0 | 7.2 |

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 60.0
 - Length (ft) = 130.0
 - Eave Height (ft) = 22.0/ 22.0
 - Roof Slope (rise/12) = 4.0/ 4.0
 - Dead Load (psf) = 3.5
 - Collateral Load (psf) = 5.0
 - Live Load (psf) = 20.0
 - Min. Uniform Roof Snow Load (psf) = 150.0
 - Ground Snow Load (psf) (Pg) = 120.0
 - Roof Snow Load (psf) (Pt) = 105.0
 - Wind Speed (mph) = 115.0
 - Wind Code = IBC 18
 - Exposure = C
 - Closure = Enclosed
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = D
 - Seismic Coeff (Fa/Sa) = 0.63

Pf = 0.7 Ce Ct I Pg
 Ce = 1
 Ct = 1.0
 I = 1
 Pg = 150.0 PSF
 Pf = 105.00 PSF
UNBALANCED LOAD = 160.4 PSF

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES THE VERTICAL COMPONENT OF BRACING REACTION IS INCLUDED IN VERTICAL REACTIONS.

| Frm Line | Col Line | Load Id | Column_Reactions(k) | | | | Bolt Qty | Dia | Base_Plate(in) | | | Grout (in) | |
|----------|----------|---------|---------------------|------|------|-------|----------|-----|----------------|--------|-------|------------|-----|
| | | | Hmax | V | Hmin | V | | | Width | Length | Thick | | |
| 2* | E | 7 | 26.4 | 64.8 | 2 | -4.4 | -5.1 | 4 | 1.000 | 8.000 | 11.00 | 0.500 | 0.0 |
| | | 8 | 23.0 | 66.1 | 4 | -0.6 | -5.8 | | | | | | |
| 2* | B | 3 | 3.7 | -2.1 | 7 | -26.4 | 66.3 | 4 | 1.000 | 8.000 | 11.00 | 0.750 | 0.0 |
| | | 1 | -22.5 | 90.0 | 6 | -0.6 | -7.6 | | | | | | |

RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead | | Collateral | | Live | | Snow | | Wind_Left1 | | Wind_Right1 | |
|------------|-------------|------|------|------------|------|------|------|-------|------|------------|-------|-------------|-------|
| | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert |
| 2* | E | 1.2 | 3.8 | 1.1 | 2.6 | 4.0 | 9.7 | 20.7 | 50.7 | -8.5 | -12.3 | -0.8 | -8.2 |
| 2* | B | -1.2 | 5.0 | -1.1 | 2.8 | -3.9 | 14.7 | -20.3 | 82.2 | -4.3 | -8.4 | 3.5 | -14.2 |

| Frame Line | Column Line | Wind_Left2 | | Wind_Right2 | | Wind_Long1 | | Wind_Long2 | | Seismic_Left | | Seismic_Right | |
|------------|-------------|------------|------|-------------|------|------------|-------|------------|-------|--------------|------|---------------|------|
| | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert |
| 2* | E | -6.0 | -6.0 | 1.6 | -1.9 | -2.2 | -13.4 | -3.4 | -12.7 | -3.7 | -2.8 | 3.7 | 2.8 |
| 2* | B | -0.4 | -2.6 | 7.4 | -8.4 | -3.2 | -13.4 | -4.4 | -14.2 | -4.1 | 3.3 | 4.2 | -2.4 |

| Frame Line | Column Line | Seismic_Long | | -MIN_SNOW-- | | F1UNB_SL_L- | | F1UNB_SL_R- | |
|------------|-------------|--------------|-------|-------------|------|-------------|------|-------------|------|
| | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert |
| 2* | E | 0.0 | -8.9 | 24.2 | 58.5 | 20.7 | 59.8 | 17.8 | 29.6 |
| 2* | B | 0.0 | -14.7 | -24.2 | 58.5 | -20.7 | 33.0 | -17.8 | 50.8 |

ENDWALL COLUMN:

MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | Bolt Qty | Dia | Base_Plate(in) | | | Grout (in) | | |
|----------|----------|---------------------|------|------|------|----------|------|----------------|--------|-------|------------|-------|-----|
| | | Load Id | Hmax | V | Hmin | | | Width | Length | Thick | | | |
| 1 | E | 9 | 1.6 | -2.4 | 10 | -1.4 | -1.7 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 11 | 1.2 | 9.9 | 5 | 0.0 | -2.7 | | | | | | |
| 1 | D | 12 | 3.8 | -5.7 | 13 | -3.4 | -2.8 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 14 | 0.0 | 33.9 | 12 | 3.8 | -5.7 | | | | | | |
| 1 | C | 12 | 3.8 | -2.7 | 10 | -3.4 | -1.7 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 |
| | | 15 | 0.0 | 28.3 | 12 | 3.8 | -2.7 | | | | | | |
| 1 | B | 12 | 1.6 | -1.6 | 10 | -1.4 | -0.9 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 1 | 0.0 | 25.9 | 12 | 1.6 | -1.6 | | | | | | |
| 9 | B | 12 | 1.6 | -2.7 | 13 | -1.4 | -0.9 | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |
| | | 1 | 0.0 | 26.1 | 12 | 1.6 | -2.7 | | | | | | |
| 9 | C | 9 | 3.8 | -3.8 | 10 | -3.4 | -1.4 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 16 | 0.0 | 28.2 | 9 | 3.8 | -3.8 | | | | | | |
| 9 | D | 12 | 3.8 | -6.0 | 13 | -3.4 | -1.4 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 |
| | | 17 | 0.0 | 33.9 | 12 | 3.8 | -6.0 | | | | | | |
| 9 | E | 18 | 1.6 | -1.4 | 10 | -1.4 | -1.4 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 |
| | | 7 | 0.0 | 9.2 | 18 | 1.6 | -1.4 | | | | | | |

ANCHOR BOLT SUMMARY

| Qty | Locate | Dia (in) | Type | Proj (in) |
|-----|---------|----------|------|-----------|
| 4 | Jamb | 1/2" | GR36 | 1.00 |
| 32 | Endwall | 3/4" | GR36 | 2.00 |
| 56 | Frame | 1" | GR36 | 2.00 |

BUILDING BRACING REACTIONS

| Wall Loc | Col Line | ± Reactions(k) | Wind | | Seismic | | Panel Shear (lb/ft) | | | | | | | | | | |
|----------|----------|----------------|------|------|---------|------|---------------------|-----|-----|-----|------|------|-----|-----|-----|------|------|
| | | | Horz | Vert | Horz | Vert | | | | | | | | | | | |
| L_SW | E,D | 4.6 | 6.3 | 3.1 | 4.3 | 11.8 | 14.7 | | | | | | | | | | |
| | | | | | | | | 5.6 | 2.8 | 3.5 | 11.8 | 14.7 | | | | | |
| | | | | | | | | | | | | | 7.8 | 2.8 | 3.5 | 11.8 | 14.7 |
| | | | | | | | | | | | | | | | | | |
| R_SW | E | 8.7 | 2.1 | 2.7 | 7.1 | 8.9 | | | | | | | | | | | |
| | | | | | | | 6.5 | 2.1 | 2.7 | 7.1 | 8.9 | | | | | | |
| | | | | | | | | | | | | 4.3 | 2.1 | 2.7 | 7.1 | 8.9 | |

Reactions for seismic represent shear force, Eh

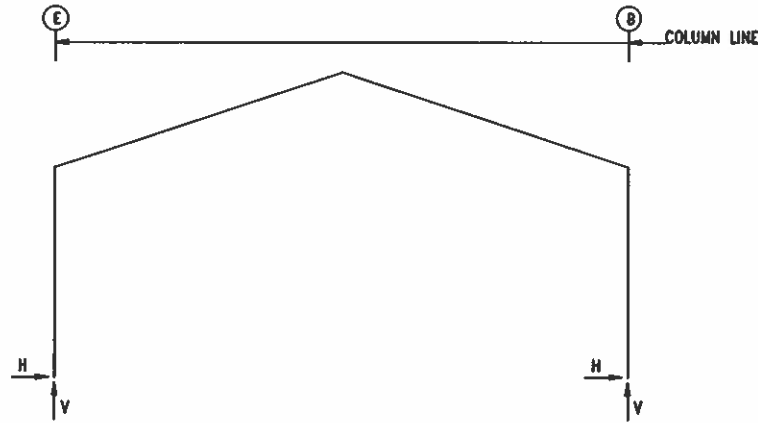
REACTIONS W/ (1) LEAN TO ATTACHED @ GRID 'B'



R & M STEEL COMPANY
 P.O. Box 580
 Caldwell, Idaho 83606
 208-454-1800 Fax 208-454-1801

| | | |
|---------------|--------------|----------------|
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| DRAWN BY | | DRAWING NUMBER |
| CAMP PINWOOD | | SB |
| CAMP PINWOOD | | OF |

FRAME LINES: 2 3 4 5 6 7 8



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES
THE VERTICAL COMPONENT OF BRACING REACTION IS INCLUDED IN VERTICAL REACTIONS.

| Frm Line | Col Line | Load Id | Column_Reactions(k) | | | | | | | | Base_Plate(in) Width | Length | Thick | Grout (in) |
|----------|--------------|---------|---------------------|------|---------|-------|------|--------------|-------|-------|----------------------|--------|-------|------------|
| | | | Hmax | Vmax | Load Id | Hmin | Vmin | Bolt(in) Qty | Dia | | | | | |
| 2* | E | 5 | 26.5 | 64.8 | 1 | -3.0 | -1.4 | 4 | 1.000 | 8.000 | 11.00 | 0.500 | 0.0 | |
| 2* | B | 2 | 3.0 | -1.4 | 5 | -26.5 | 64.8 | 4 | 1.000 | 8.000 | 11.00 | 0.500 | 0.0 | |
| 2* | | 5 | -26.5 | 64.8 | 4 | -1.0 | -4.3 | | | | | | | |
| 2* | Frame lines: | | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | | |

RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead | | Collateral | | Live | | Snow | | Wind_Left1 | | Wind_Right1 | |
|------------|-------------|------|------|------------|------|------|------|-------|------|------------|-------|-------------|-------|
| | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert |
| 2* | E | 1.2 | 3.8 | 1.1 | 2.6 | 4.0 | 9.8 | 21.2 | 51.2 | -6.1 | -10.2 | 0.9 | -6.7 |
| 2* | B | -1.2 | 3.8 | -1.1 | 2.6 | -4.0 | 9.8 | -21.2 | 51.2 | -0.9 | -6.7 | 6.1 | -10.2 |

BUILDING BRACING REACTIONS

| Wall Loc | Col Line | E | D | ± Reactions(k) | | Panel_Shear (lb/ft) |
|----------|----------|-----|-----|----------------|---------|---------------------|
| | | | | Wind | Seismic | |
| L_EW | 1 | 2.3 | 3.1 | 3.1 | 4.3 | |
| F_SW | B | 3.4 | 2.1 | 2.7 | 7.1 | 8.9 |
| | | 5.6 | 2.1 | 2.7 | 7.1 | 8.9 |
| | | 7.8 | 2.1 | 2.7 | 7.1 | 8.9 |
| R_EW | 9 | 2.3 | 3.0 | 3.1 | 4.1 | |
| B_SW | E | 8.7 | 2.1 | 2.7 | 7.1 | 8.9 |
| | | 6.5 | 2.1 | 2.7 | 7.1 | 8.9 |
| | | 4.3 | 2.1 | 2.7 | 7.1 | 8.9 |

Reactions for seismic represent shear force, Eh

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

| Frm Line | Col Line | Dead | Collat | Live | Snow | Wind_Left1 | | Wind_Right1 | | Wind_Left2 | | Wind_Right2 | | Wind Press |
|----------|----------|------|--------|------|------|------------|------|-------------|------|------------|------|-------------|------|------------|
| | | | | | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | |
| 1 | E | 0.5 | 0.4 | 1.4 | 7.3 | -2.3 | -5.1 | 0.0 | 1.8 | -2.3 | -4.1 | 0.0 | 2.7 | -2.3 |
| 1 | D | 1.4 | 1.0 | 3.8 | 20.0 | 0.0 | -2.1 | 2.3 | -6.8 | 0.0 | -0.9 | 2.3 | -5.6 | -5.7 |
| 1 | C | 1.4 | 1.0 | 3.8 | 20.0 | 0.0 | -2.8 | 0.0 | -5.3 | 0.0 | -1.6 | 0.0 | -4.2 | -5.7 |
| 1 | B | 0.5 | 0.4 | 1.4 | 7.3 | 0.0 | -2.2 | 0.0 | -1.8 | 0.0 | -1.3 | 0.0 | -0.8 | -2.3 |

| Frm Line | Col Line | Dead | Collat | Live | Snow | Wind_Left1 | | Wind_Right1 | | Wind_Left2 | | Wind_Right2 | | Wind Press |
|----------|----------|------|--------|------|------|------------|------|-------------|------|------------|------|-------------|------|------------|
| | | | | | | Horz | Vert | Horz | Vert | Horz | Vert | Horz | Vert | |
| 9 | B | 0.5 | 0.4 | 1.4 | 7.3 | 0.0 | -1.8 | 0.0 | -2.1 | 0.0 | -0.8 | 0.0 | -1.1 | -2.3 |
| 9 | C | 1.4 | 1.0 | 3.8 | 20.0 | -2.3 | -8.4 | 0.0 | 0.1 | -2.3 | -7.2 | 0.0 | 1.3 | -5.7 |
| 9 | D | 1.4 | 1.0 | 3.8 | 20.0 | 0.0 | 0.1 | 2.3 | -8.4 | 0.0 | 1.3 | 2.3 | -7.2 | -5.7 |
| 9 | E | 0.5 | 0.4 | 1.4 | 7.3 | 0.0 | -2.1 | 0.0 | -1.8 | 0.0 | -1.1 | 0.0 | -0.8 | -2.3 |

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

| Frm Line | Col Line | Column_Reactions(k) | | | | | | | | Bolt(in) Qty | Dia | Base_Plate(in) | | Thick | Grout (in) |
|----------|----------|---------------------|------|------|---------|------|------|-------|--------|--------------|-------|----------------|-----|-------|------------|
| | | Load Id | Hmax | Vmax | Load Id | Hmin | Vmin | Width | Length | | | | | | |
| 1 | E | 6 | 1.6 | -2.8 | 7 | -1.4 | -1.7 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 | | |
| 1 | D | 8 | 3.8 | -3.2 | 9 | -3.4 | -2.8 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 | | |
| 1 | C | 8 | 3.8 | -2.3 | 7 | -3.4 | -1.7 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 | | |
| 1 | B | 12 | 1.6 | -1.4 | 7 | -1.4 | -1.4 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 | | |
| 9 | B | 13 | 1.6 | -1.4 | 9 | -1.4 | -1.4 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 | | |
| 9 | C | 6 | 3.8 | -4.2 | 7 | -3.4 | -1.4 | 4 | 0.750 | 6.000 | 12.00 | 0.375 | 0.0 | | |
| 9 | D | 14 | 0.0 | 28.2 | 6 | 3.8 | -4.2 | | | | | | | | |
| 9 | E | 12 | 1.6 | -1.4 | 7 | -1.4 | -1.4 | 4 | 0.750 | 6.500 | 6.000 | 0.250 | 0.0 | | |

REACTIONS W/ NO LEAN TO'S ATTACHED

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 Width (ft) = 60.0
 Length (ft) = 130.0
 Eave Height (ft) = 22.0 / 22.0
 Roof Slope (rise/12) = 4.0 / 4.0
 Dead Load (psf) = 3.5
 Collateral Load (psf) = 5.0
 Live Load (psf) = 20.0
 Min. Uniform Roof Snow Load (psf) = 150.0
 Ground Snow Load (psf) (Pg) = 120.0
 Roof Snow Load (psf) (Pf) = 105.0
 Wind Speed (mph) = 115.0
 Wind Code = IBC 18
 Exposure = C
 Closure = Enclosed
 Importance Wind = 1.00
 Importance Seismic = 1.00
 Seismic Zone = 0
 Seismic Coeff (Fa*Ss) = 0.63
- Loading conditions are:
 1 0.6Dead+0.6Wind_Left2
 2 0.6Dead+0.6Wind_Right2
 3 0.6Dead+0.6Wind_Long1L
 4 0.6Dead+0.6Wind_Long2L
 5 Dead+Collateral+MIN_SNOW
 6 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
 7 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
 8 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
 9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
 10 Dead+Collateral+E1UNB_SL_L
 11 Dead+Collateral+E1UNB_SL_R
 12 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
 13 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
 14 Dead+Collateral+E2UNB_SL_L
 15 Dead+Collateral+E2UNB_SL_R

| |
|------------------------------------|
| Pf = 0.7 Ce Cl I Pg |
| Ce = 1 |
| Cl = 1.0 |
| I = 1 |
| Pg = 150.0 PSF |
| Pf = 105.00 PSF |
| UNBALANCED LOAD = 160.4 PSF |

ANCHOR BOLT SUMMARY

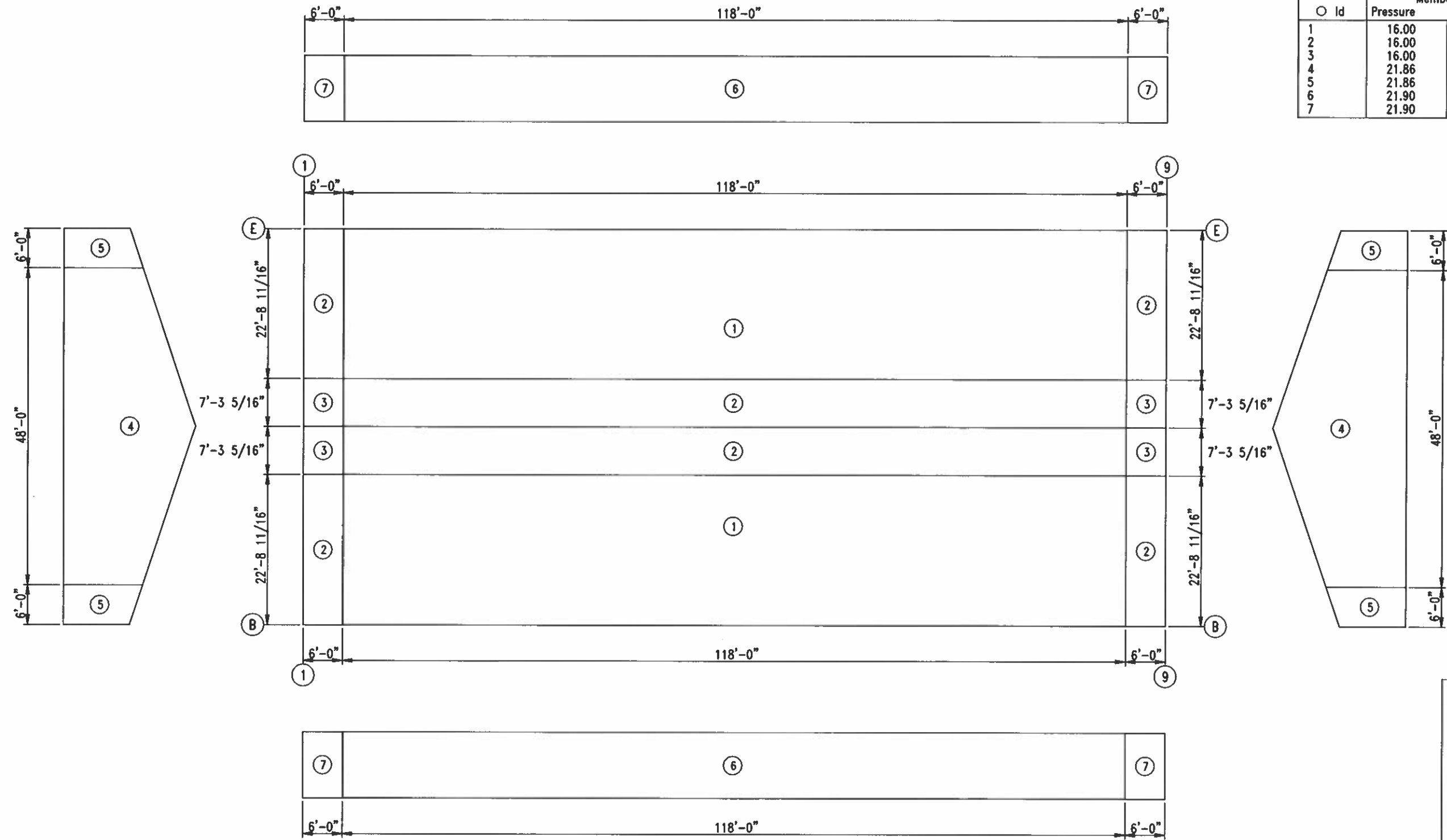
| Qty | Locate | Dia (in) | Type | Proj (in) |
|-----|---------|----------|------|-----------|
| 4 | Jamb | 1/2" | GR36 | 1.00 |
| 32 | Endwall | 3/4" | GR36 | 2.00 |
| 56 | Frame | 1" | GR36 | 2.00 |




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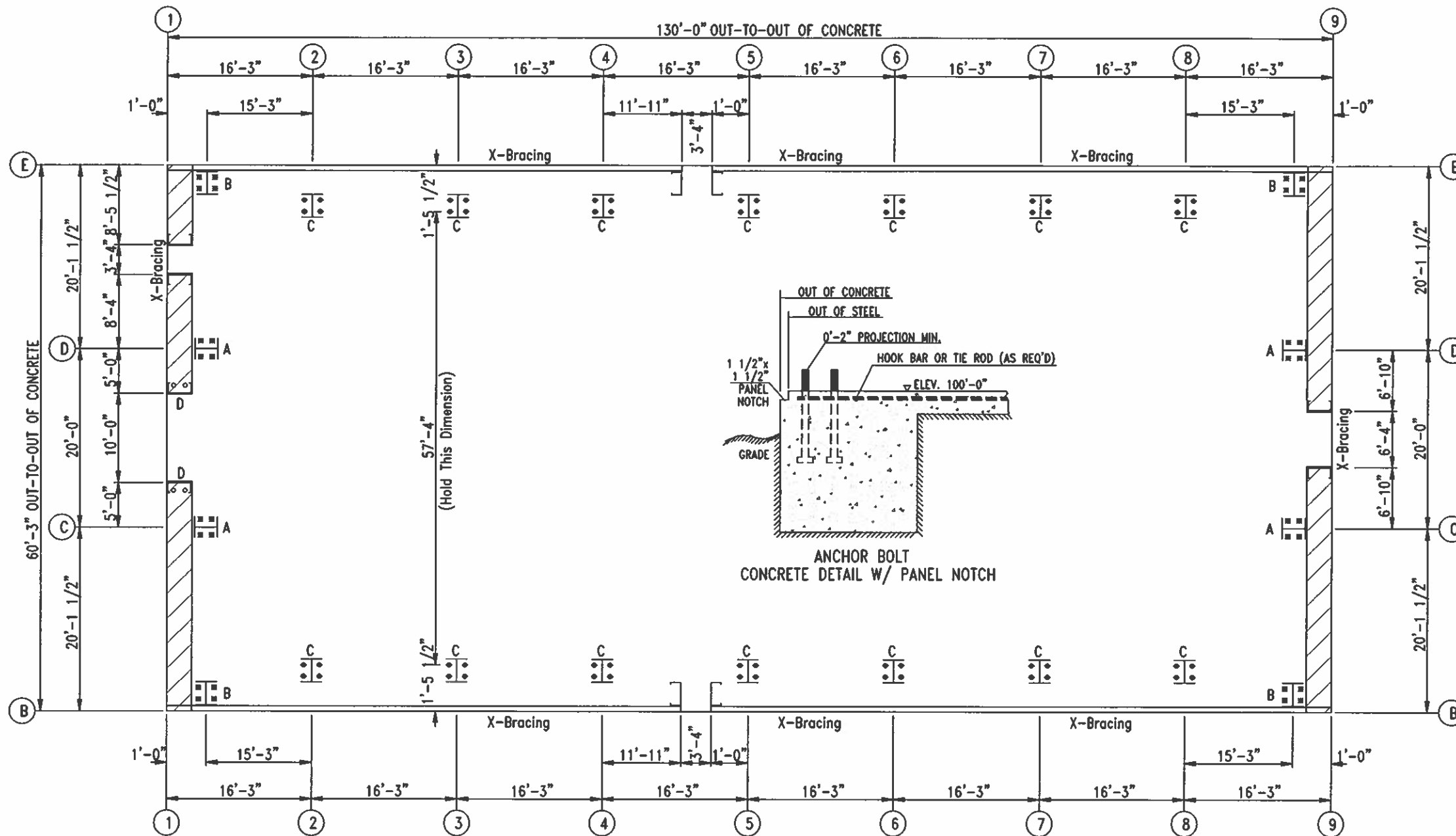
| | | |
|----------------|--------------|----------|
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| DRAWING NUMBER | | DRAWN BY |
| CAMP PINEWOOD | | SB |
| CAMP PINEWOOD | | OF |

| COMPONENTS AND CLADDING (Unfactored) | | | | |
|--------------------------------------|----------|---------|----------|---------|
| ○ Id | Member | | Panel | |
| | Pressure | Suction | Pressure | Suction |
| 1 | 16.00 | -16.00 | 16.46 | -50.15 |
| 2 | 16.00 | -30.40 | 16.46 | -73.22 |
| 3 | 16.00 | -45.60 | 16.46 | -86.76 |
| 4 | 21.86 | -24.16 | 27.15 | -36.35 |
| 5 | 21.86 | -25.85 | 27.15 | -36.35 |
| 6 | 21.90 | -24.20 | 27.10 | -36.30 |
| 7 | 21.90 | -25.89 | 27.10 | -36.30 |

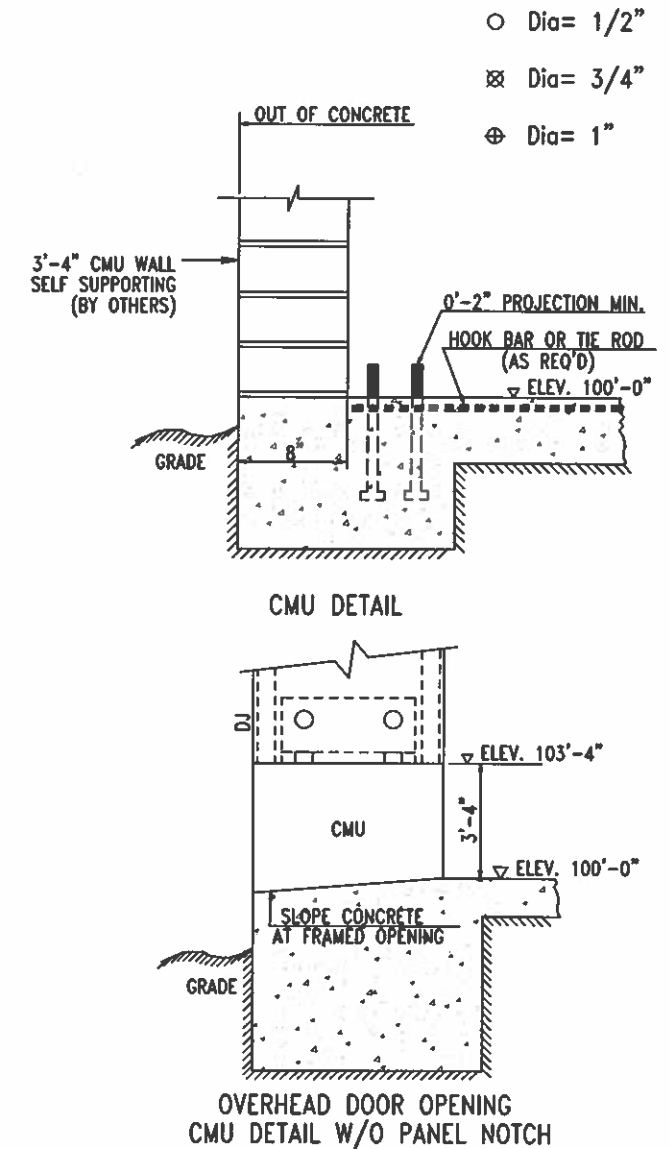


PANEL ZONE LAYOUT
(Wind Pressures, Unfactored (psf))

| | | |
|--|---------------|-------------------|
|  R & M STEEL COMPANY P.O. Box 580 Caldwell, Idaho 83606 208-454-1800 Fax 208-454-1801 | | |
| SCALE: | JOB LOCATION: | REVISION: |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINWOOD | | DRAWN BY: SB |
| CAMP PINWOOD | | DRAWING NUMBER OF |

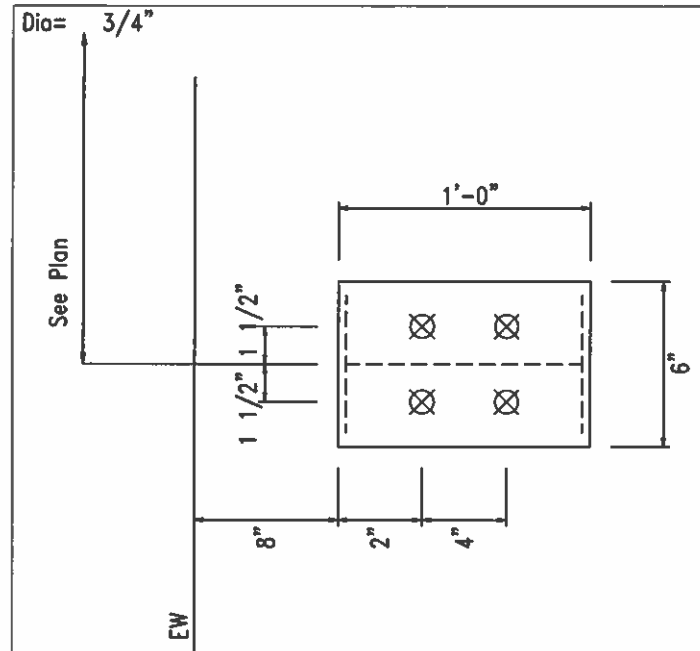


ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)

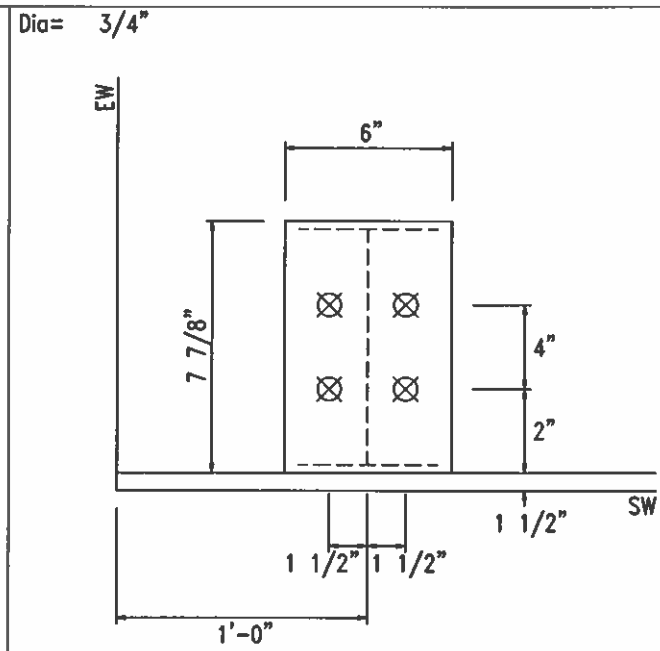


**NOT FOR
CONSTRUCTION**

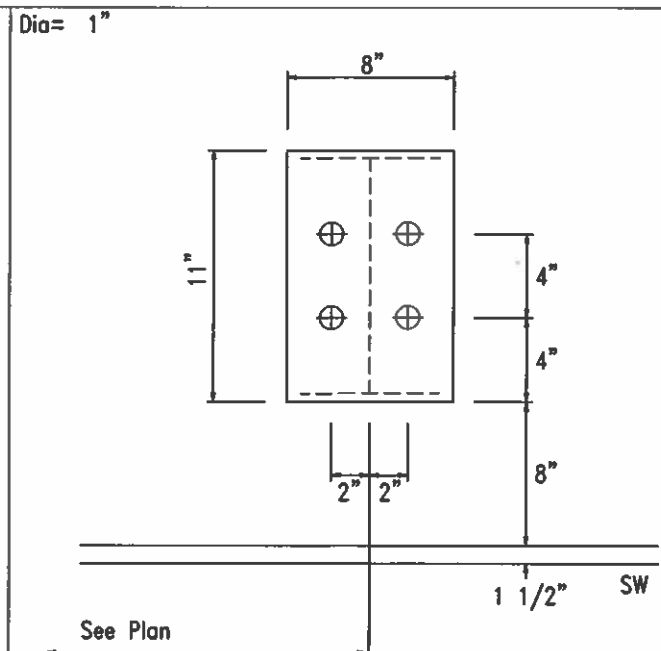
| | |
|--|----------------|
| R & M STEEL COMPANY | |
| P.O. Box 580 Coldwell, Idaho 83606 208-454-1800 Fax 208-454-1801 | |
| SCALE: | JOB LOCATION |
| DATE: 2/14/24 | MCCALL, ID |
| REVISION | DRAWN BY |
| CAMP PINWOOD | SB |
| CAMP PINWOOD | DRAWING NUMBER |
| OF | OF |



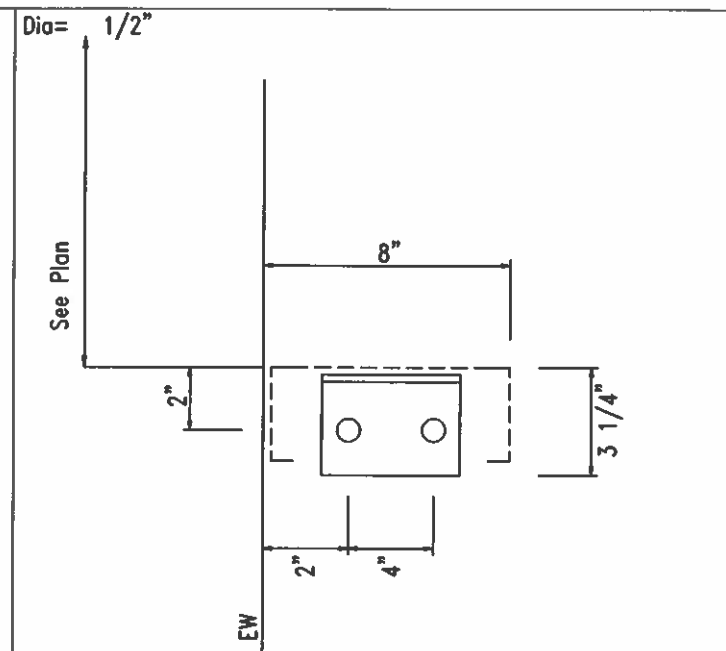
DETAIL A



DETAIL B



DETAIL C



DETAIL D

Base EL. 103'-4"



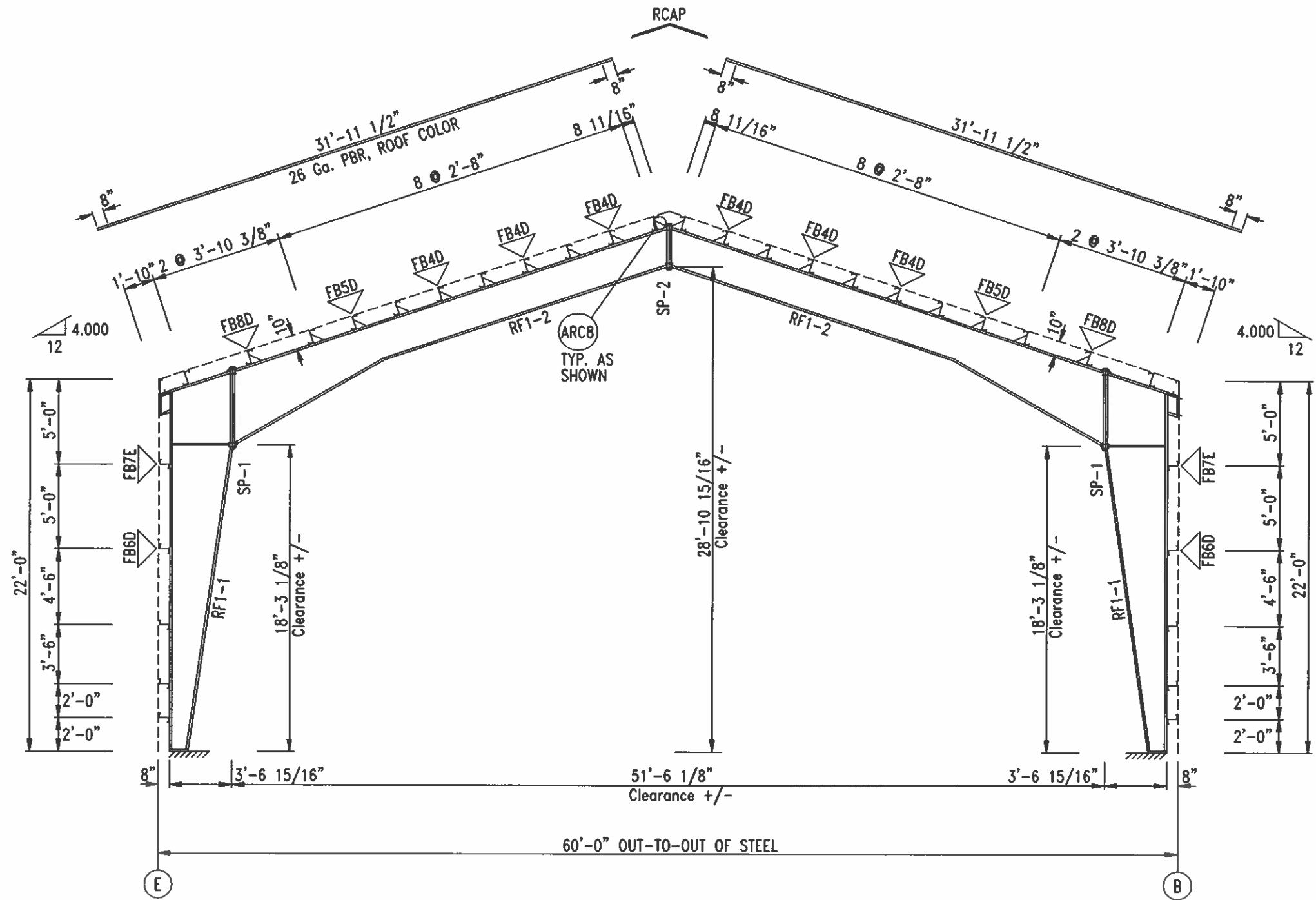
R & M STEEL COMPANY
 P.O. Box 580
 Caldwell, Idaho 83606
 208-454-1800 Fax 208-454-1801

| | | |
|---------------|--------------|----------------------|
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINEWOOD | | DRAWN BY SB |
| CAMP PINEWOOD | | DRAWING NUMBER OF |


| SPLICE PLATE & BOLT TABLE | | | | | | | | | |
|---------------------------|-----|-----|-----|------|-------|--------|-------|-------|-------------|
| Mark | Qty | | Int | Type | Dia | Length | Width | Thick | Length |
| | Top | Bot | | | | | | | |
| SP-1 | 4 | 4 | 0 | A325 | 0.875 | 2.75 | 6" | 3/4" | 4'-9 13/16" |
| SP-2 | 4 | 4 | 0 | A325 | 0.750 | 2.00 | 6" | 1/2" | 2'-8 1/4" |

| MEMBER TABLE | | | | | | |
|--------------|-----------|-------|-----------|-------|------------------------------------|-----------------------------------|
| Mark | Web Depth | | Web Plate | | Outside Flange W x Thk x Length | Inside Flange W x Thk x Length |
| | Start/End | Thick | Length | Thick | | |
| RF1-1 | 10.0/14.3 | 0.250 | 28.8 | 0.250 | 6 x 1/4" x 254.7 | 6 x 3/4" x 217.0 |
| RF1-2 | 14.3/42.0 | 0.250 | 240.0 | 0.250 | 6 x 5/8" x 52.3 | 6 x 3/8" x 122.3 |
| | 48.0/24.0 | 0.250 | 120.0 | 0.179 | 6 x 1/2" x 324.3 | 6 x 3/8" x 212.3 |

FLANGE BRACES: Both Sides(U.N.)
 FBxD(1)
 D - L2x2x14
 E - L2x2x12



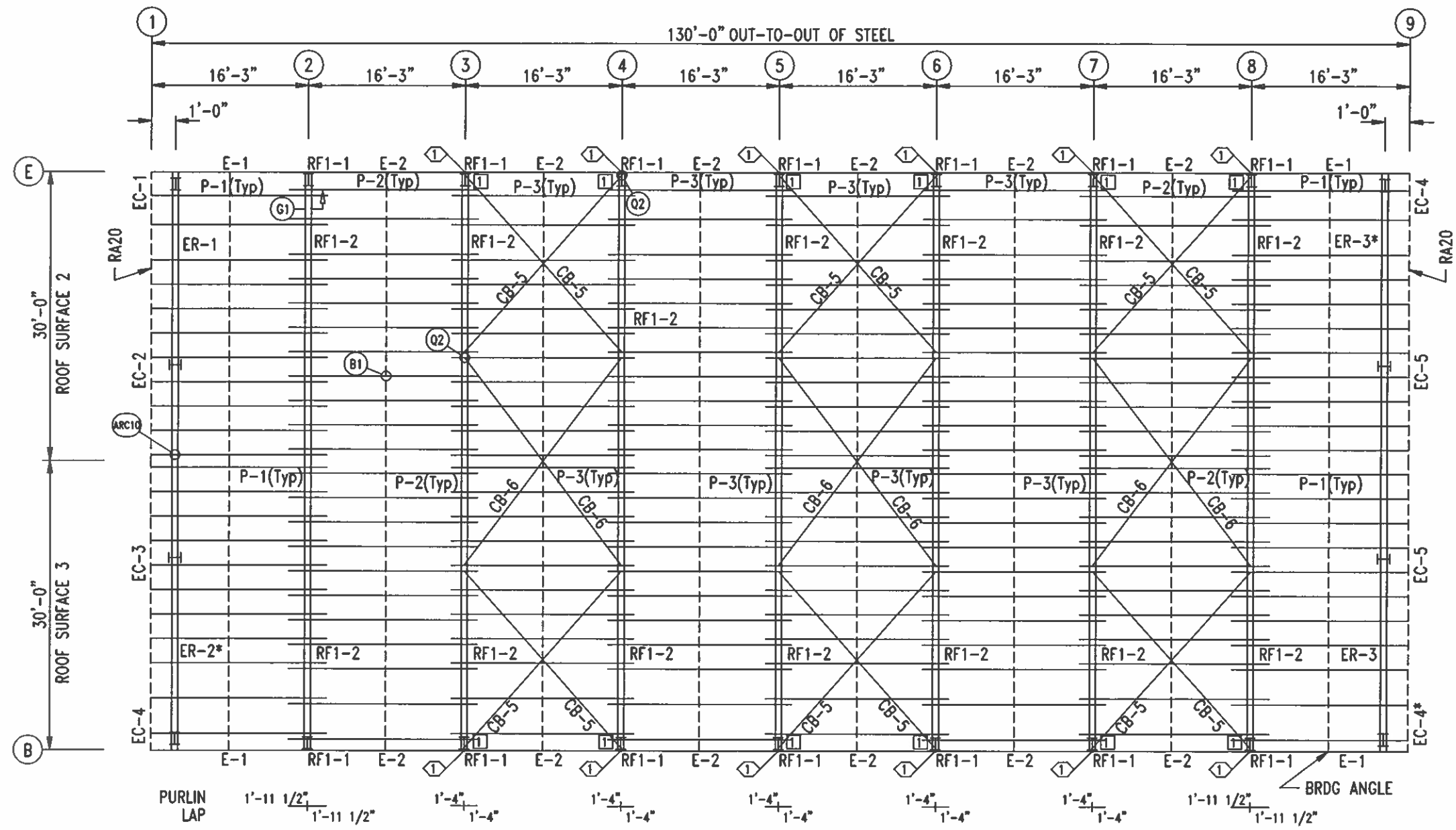
RIGID FRAME CROSS SECTION: FRAME LINE 2 3 4 5 6 7 8

| | | |
|--|--------------|----------------------|
|  R & M STEEL COMPANY P.O. Box 580 Caldwell, Idaho 83606 208-454-1800 Fax 208-454-1801 | | |
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINWOOD | | DRAWN BY SB |
| CAMP PINWOOD | | DRAWING NUMBER OF |

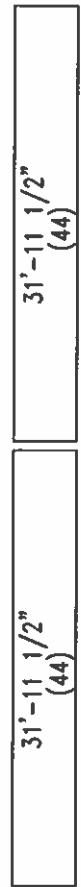
| SPECIAL BOLTS | | | | | |
|---------------|------|------|------|--------|------|
| ROOF PLAN | | | | | |
| ○ ID | QUAN | TYPE | DIA | LENGTH | WASH |
| 1 | 4 | GR_5 | 1/2" | 1 1/2" | 2 |

| MEMBER TABLE | | |
|--------------|----------|------------|
| ROOF PLAN | | |
| MARK | PART | LENGTH |
| P-1 | 10x25Z12 | 18'-2 1/2" |
| P-2 | 10x25Z14 | 19'-6 1/2" |
| P-3 | 10x25Z14 | 18'-11" |
| E-1 | 109x5E44 | 16'-2 3/4" |
| E-2 | 109x5E44 | 16'-2 1/2" |
| CB-5 | 5/16EHS | 24'-0" |
| CB-6 | 1/4EHS | 26'-0" |

| CONNECTION PLATES | |
|-------------------|-----------|
| ROOF PLAN | |
| □ ID | MARK/PART |
| 1 | EP4B4.5 |




ROOF FRAMING PLAN



ROOF SHEETING

PANELS: 26 Ga. PBR
ROOF COLOR

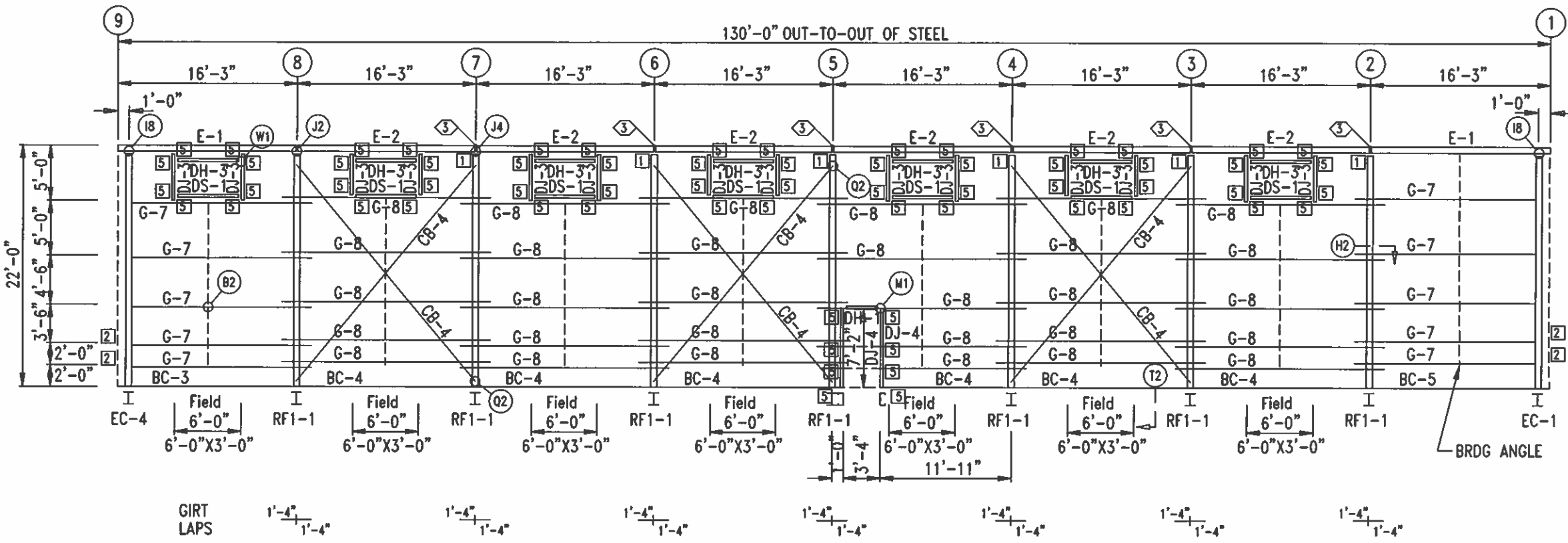
INSTALL ROOF FLANGE BRACE
AS PER RIGID FRAME CROSS
SECTION

| | | |
|--|--------------|----------------------|
|  R & M STEEL COMPANY P.O. Box 580 Caldwell, Idaho 83806 208-454-1800 Fax 208-454-1801 | | |
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINWOOD | | DRAWN BY SB |
| CAMP PINWOOD | | DRAWING NUMBER OF |

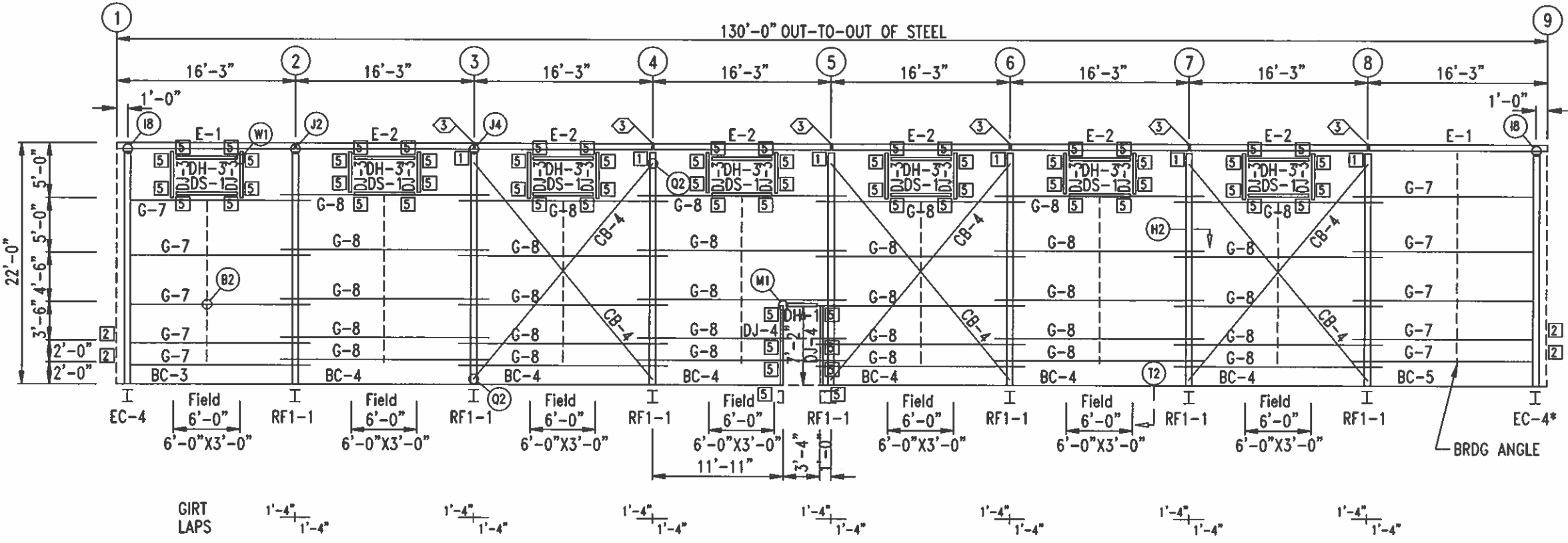
| SPECIAL BOLTS | | | | | |
|---------------|------|------|------|--------|------|
| ID | QUAN | TYPE | DIA | LENGTH | WASH |
| 3 | 6 | GR 5 | 1/2" | 1 1/2" | 0 |

| MEMBER TABLE GRID B & E | | |
|----------------------------|----------|------------|
| MARK | PART | LENGTH |
| DJ-3 | 8x25C16 | 4'-1 7/16" |
| DJ-4 | 8x25C16 | 7'-6" |
| DH-1 | 8x30U16 | 3'-4" |
| DH-3 | 8x25C16 | 6'-0" |
| DS-1 | 8x25C16 | 6'-0" |
| E-1 | 109x5E44 | 16'-2 3/4" |
| E-2 | 109x5E44 | 16'-2 1/2" |
| G-7 | 8x25Z16 | 16'-3" |
| G-8 | 8x25Z16 | 18'-11" |
| BC-3 | 8x25C16 | 14'-10" |
| BC-4 | 8x25C16 | 16'-3" |
| BC-5 | 8x25C16 | 14'-11" |
| CB-4 | 5/8EHS | 25'-0" |

| CONNECTION PLATES GRID B & E | |
|---------------------------------|-----------|
| ID | MARK/PART |
| 1 | EP484.5 |
| 2 | SA |
| 3 | CA2 |
| 4 | CA2T |
| 5 | CA7 |
| 6 | CA52 |



SIDEWALL FRAMING: GRID E



SIDEWALL FRAMING: GRID B

INSTALL SIDEWALL FLANGE BRACE
AS PER RIGID FRAME CROSS SECTION

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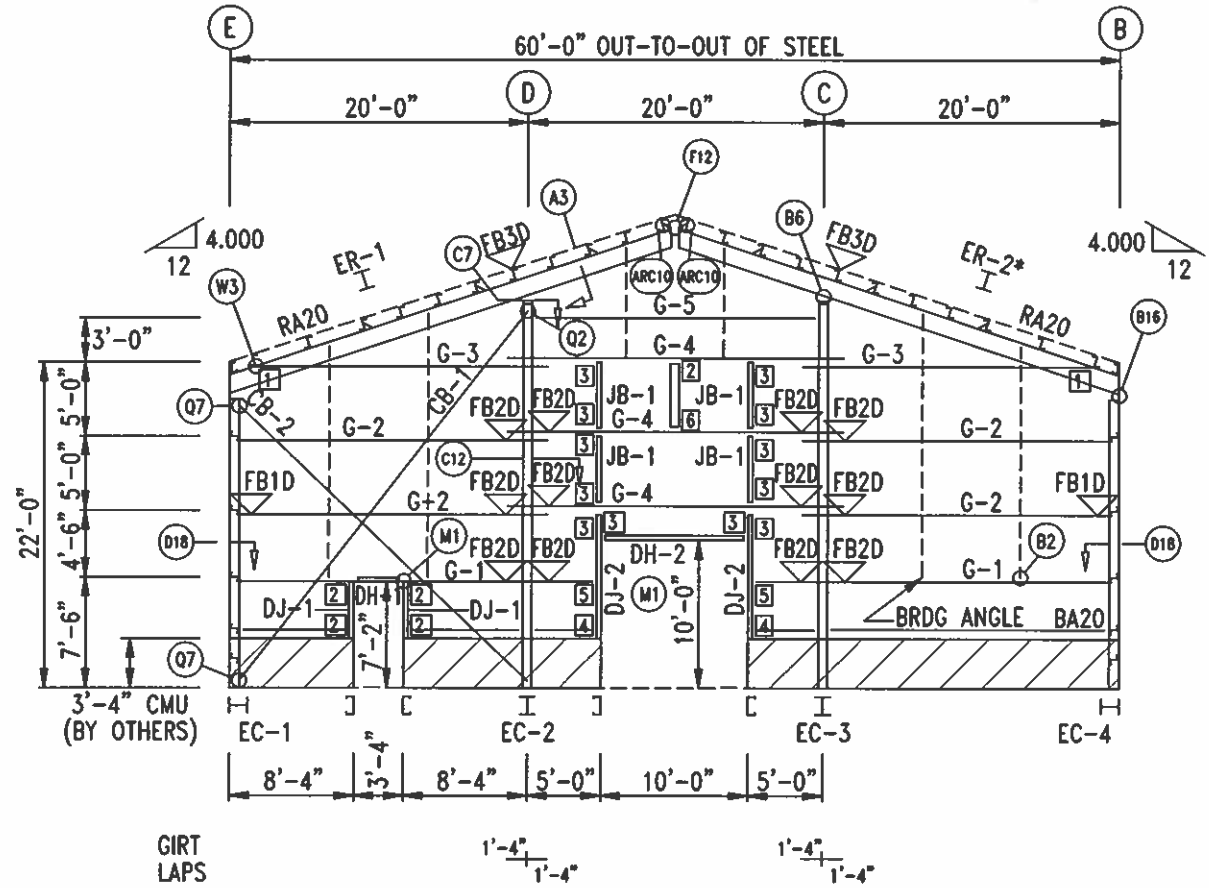
| | | |
|---------------|--------------|----------------------|
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINWOOD | | DRAWN BY SB |
| CAMP PINWOOD | | DRAWING NUMBER OF |

| BOLT TABLE GRID 1 & 9 | | | | |
|--------------------------|------|------|------|--------|
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| Cor_Column/Raf | 4 | A325 | 3/4" | 2" |
| ER-1/ER-2* | 8 | A325 | 5/8" | 1 3/4" |
| ER-3/ER-3* | 8 | A325 | 5/8" | 1 3/4" |
| Int_Column/Raf | 4 | GR 5 | 1/2" | 1" |

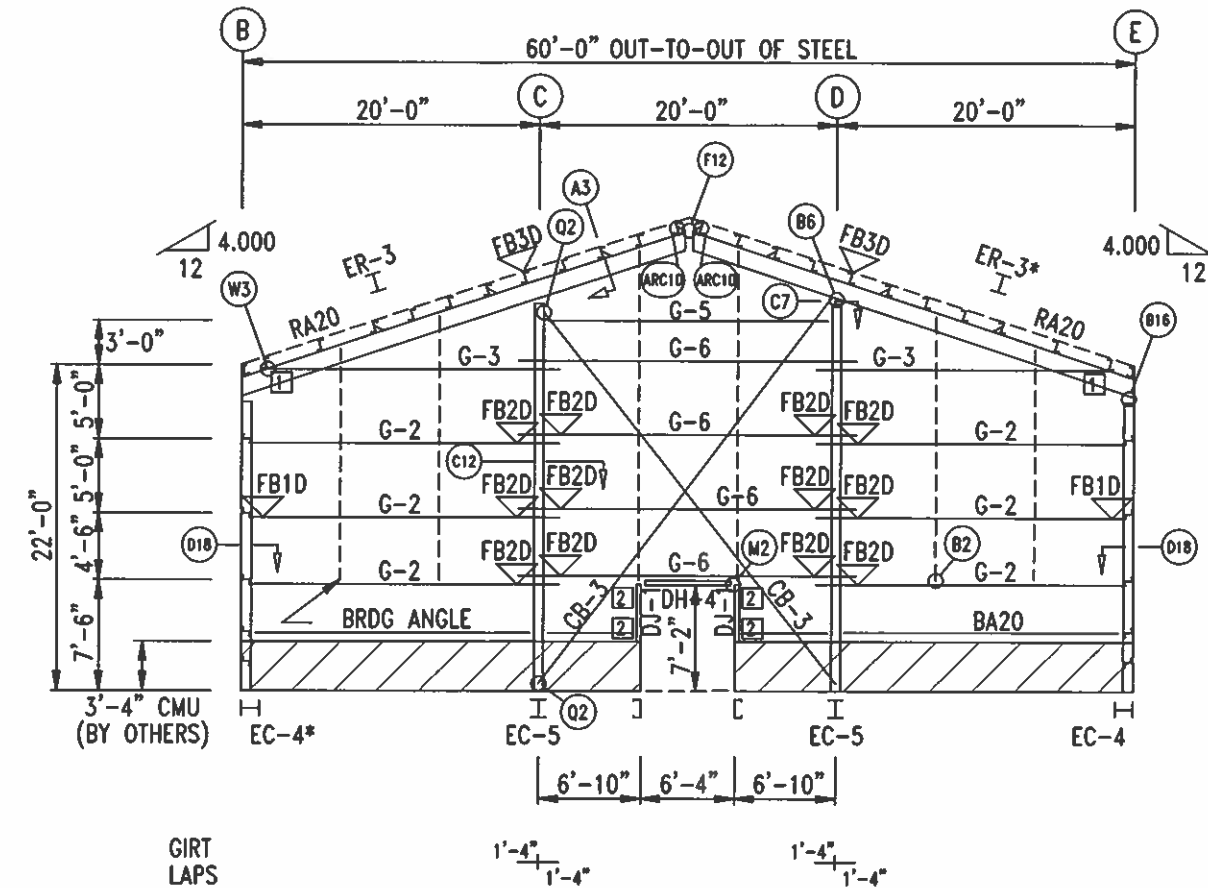
| MEMBER TABLE GRID 1 & 9 | | |
|----------------------------|---------|------------|
| MARK | PART | LENGTH |
| EC-1 | W8x10 | 20'-3 1/4" |
| EC-2 | W12x14 | 26'-8 5/8" |
| EC-3 | W12x14 | 26'-8 5/8" |
| EC-4 | W8x10 | 20'-3 1/4" |
| EC-4* | W8x10 | 20'-3 1/4" |
| EC-5 | W12x14 | 26'-8 5/8" |
| ER-1 | W12x19 | 31'-7 1/2" |
| ER-2* | W12x19 | 31'-7 1/2" |
| ER-3 | W12x19 | 31'-7 1/2" |
| ER-3* | W12x19 | 31'-7 1/2" |
| DJ-1 | 8x25C16 | 4'-2" |
| DJ-2 | 8x25C16 | 8'-8" |
| DH-1 | 8x25C16 | 3'-4" |
| DH-2 | 8x25C16 | 10'-0" |
| DH-4 | 8x30U16 | 6'-4" |
| G-1 | 8x25Z16 | 24'-8" |
| G-2 | 8x25Z16 | 21'-4" |
| G-3 | 8x25Z16 | 18'-8 3/8" |
| G-4 | 8x25Z16 | 22'-8" |
| G-5 | 8x25Z15 | 19'-4" |
| G-6 | 8x25Z16 | 22'-8" |
| CB-1 | 3/8EHS | 32'-0" |
| CB-2 | 3/8EHS | 27'-0" |
| CB-3 | 5/16EHS | 32'-0" |
| JB-1 | 8x25C16 | 5'-0" |

| FLANGE BRACE TABLE GRID 1 & 9 | | |
|----------------------------------|------|-----------|
| ID | MARK | LENGTH |
| 1 | FB3D | 1'-7 7/8" |
| 2 | FB1D | 1'-2 3/4" |
| 3 | FB2D | 1'-7 1/8" |

| CONNECTION PLATES GRID 1 & 9 | |
|---------------------------------|-----------|
| ID | MARK/PART |
| 1 | CA7 |
| 2 | CA7 |
| 3 | CA2 |
| 4 | CA2A |
| 5 | CA52 |



ENDWALL FRAMING: GRID 1



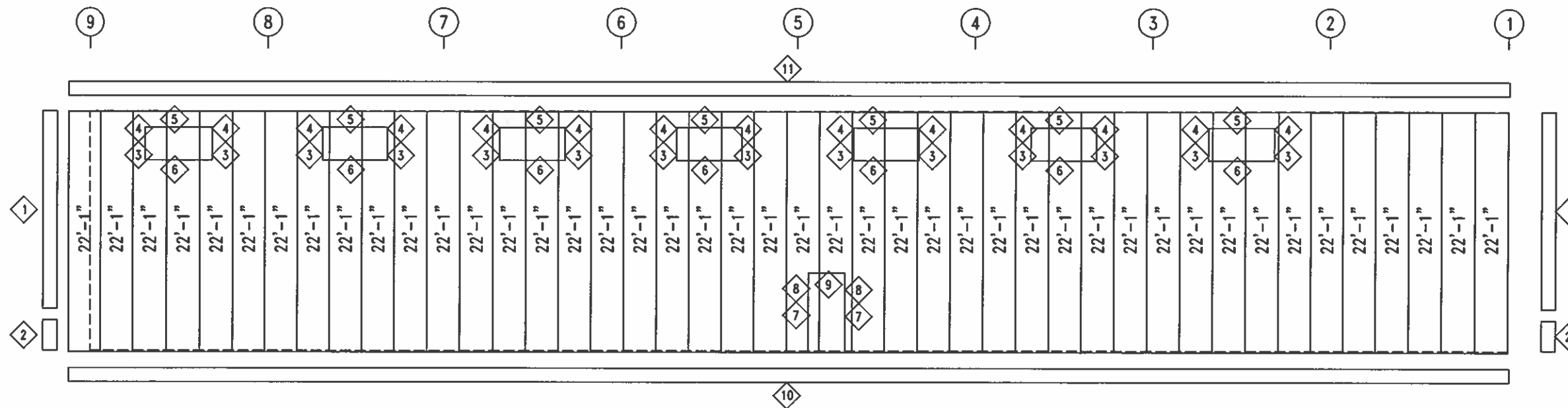
ENDWALL FRAMING: GRID 9

INSTALL ENDWALL FLANGE BRACE

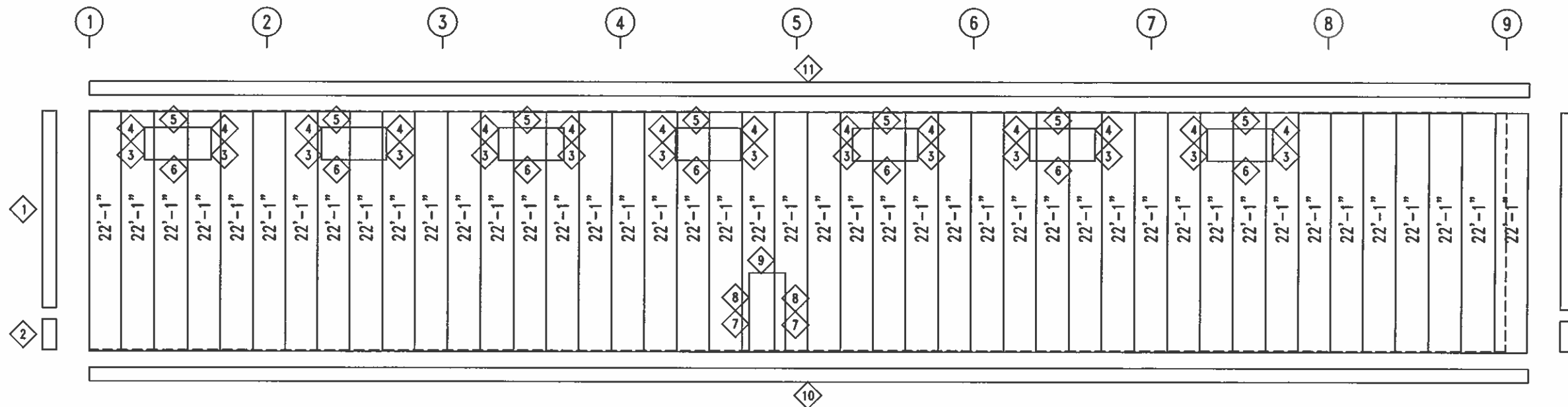
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| | | |
|---------------|--------------|----------------------|
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| CAMP PINWOOD | | DRAWN BY SB |
| CAMP PINWOOD | | DRAWING NUMBER OF |

| TRIM TABLE | | B & E |
|------------|--------|--------|
| ◇ID | MARK | LENGTH |
| 1 | S-11 | 19'-0" |
| 2 | S-58 | 3'-6" |
| 3 | JMBTRM | 3'-1" |
| 4 | JMBSTF | 3'-1" |
| 5 | HEDTRM | 7'-0" |
| 6 | SILTRM | 7'-0" |
| 7 | JMBTRM | 7'-4" |
| 8 | JMBSTF | 7'-4" |
| 9 | HEDTRM | 4'-6" |
| 10 | S-17 | 7'-4" |
| 11 | BT1 | 4'-6" |



SIDEWALL SHEETING: GRID E
 PANELS: 26 Ga. PBR - WALL COLOR

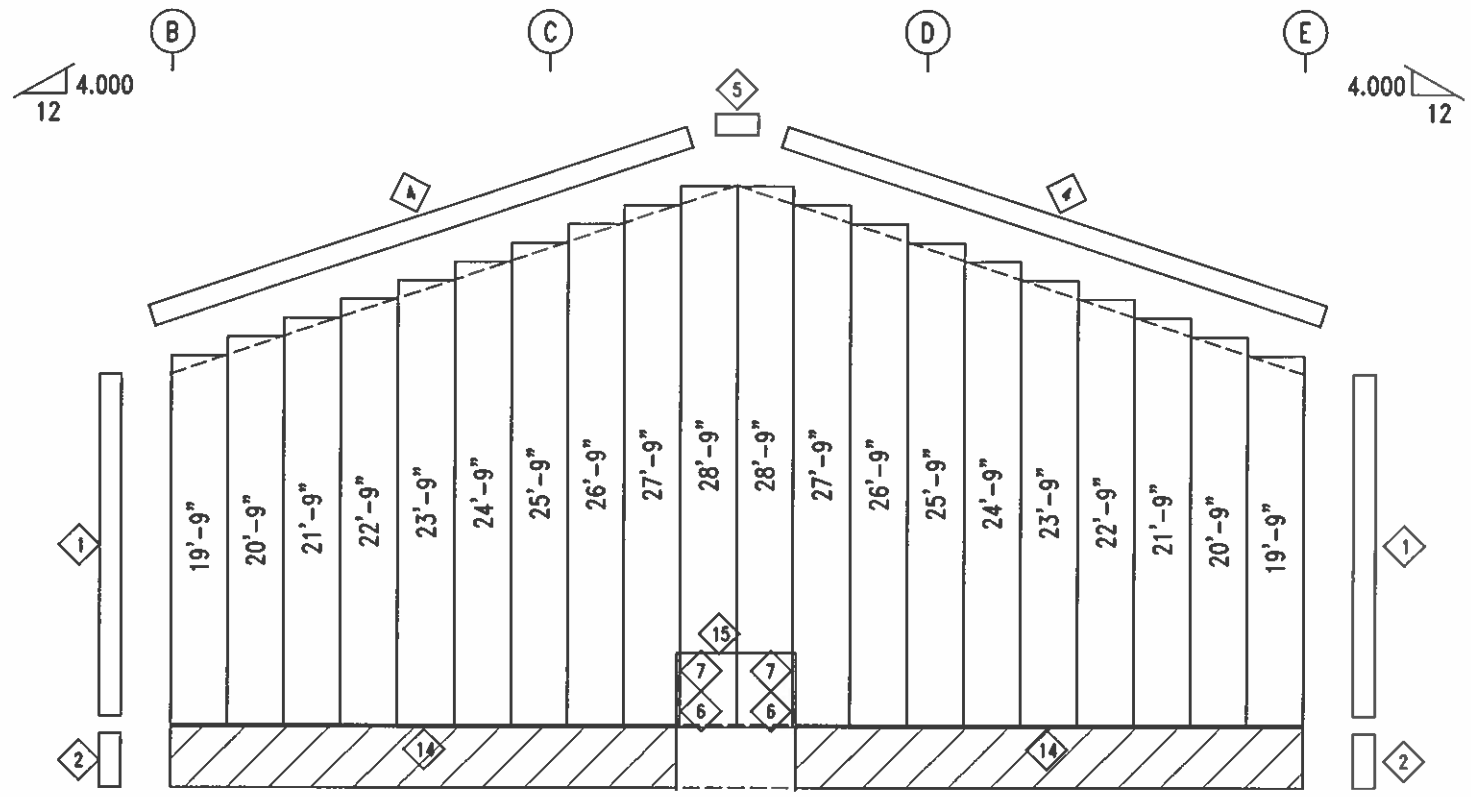


SIDEWALL SHEETING: GRID B
 PANELS: 26 Ga. PBR - WALL COLOR

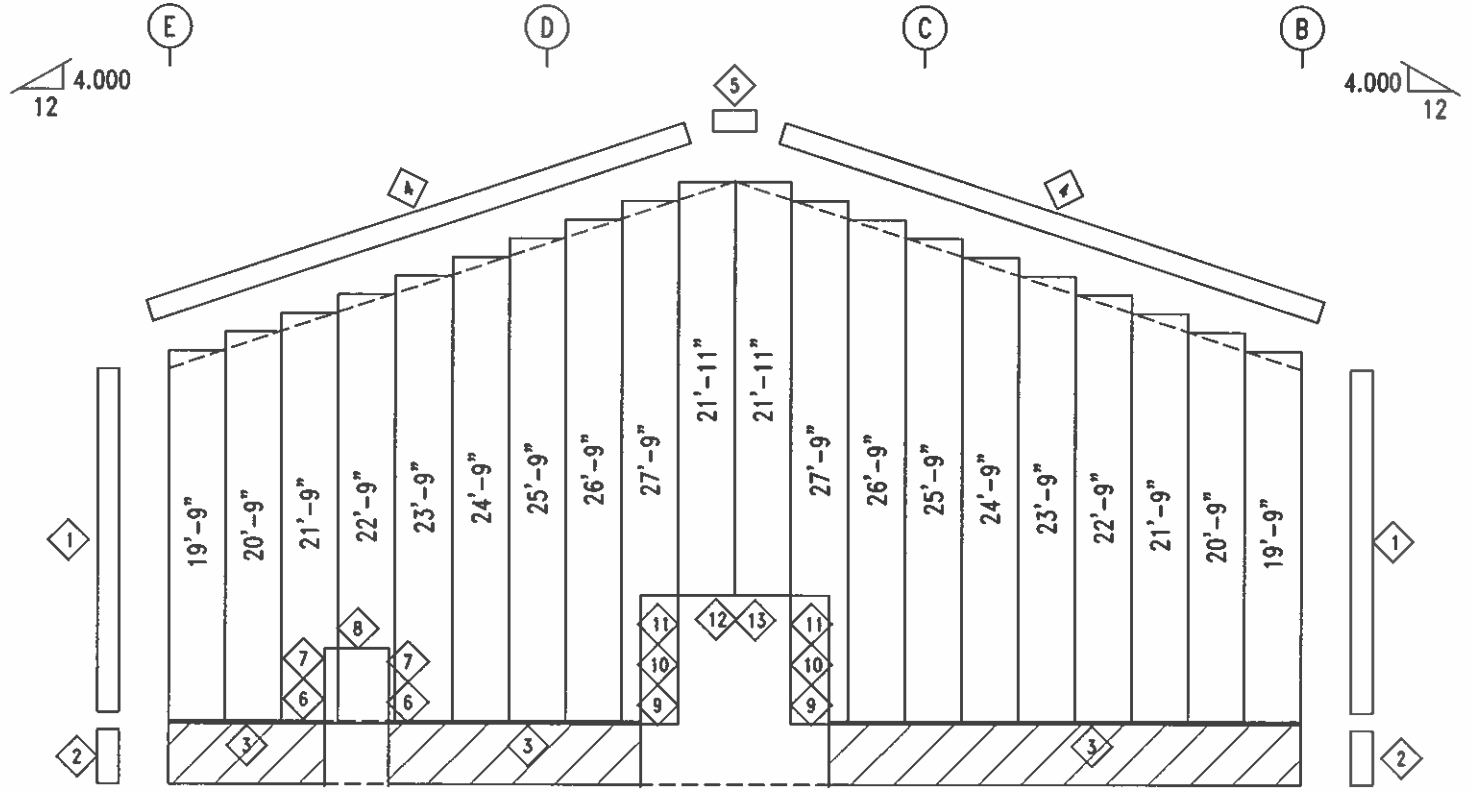


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| | | |
|----------------|--------------|----------|
| SCALE: | JOB LOCATION | REVISION |
| DATE: 2/14/24 | MCCALL, ID | |
| DRAWN BY | | |
| CAMP PINWOOD | | SB |
| DRAWING NUMBER | | |
| CAMP PINWOOD | | OF |




ENDWALL SHEETING: GRID 9
 PANELS: 26 Ga. PBR - WALL COLOR

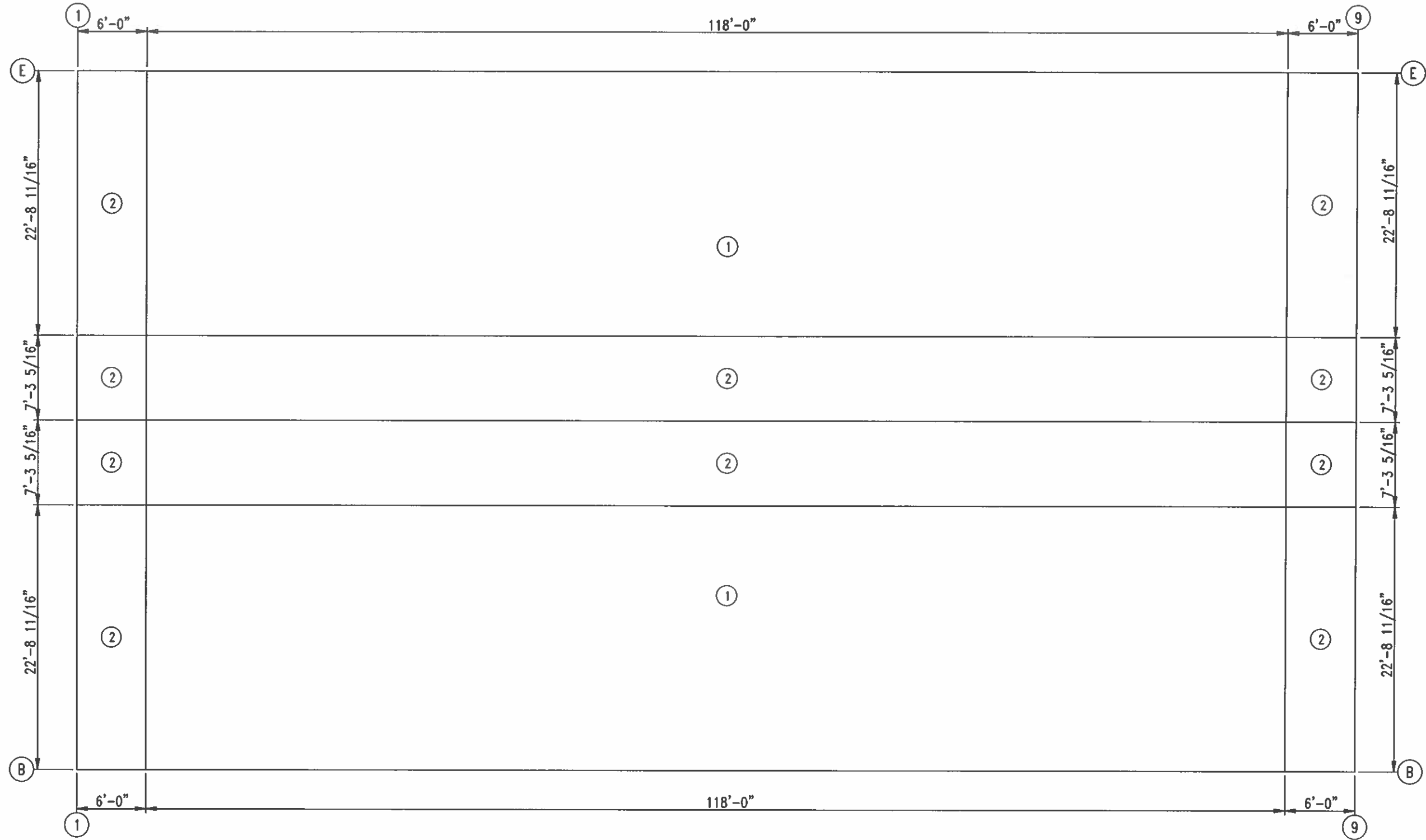


ENDWALL SHEETING: GRID 1
 PANELS: 26 Ga. PBR - WALL COLOR

| TRIM TABLE | | |
|------------|--------|--------|
| FRAME LINE | | 1 & 9 |
| ◇ID | MARK | LENGTH |
| 1 | S-11 | 19'-0" |
| 2 | S-58 | 3'-6" |
| 3 | BT2 | 27'-0" |
| 4 | S-188 | 16'-6" |
| 5 | PK BOX | 2'-6" |
| 6 | JMBTRM | 7'-4" |
| 7 | JMBSTF | 7'-4" |
| 8 | HEDTRM | 4'-6" |
| 9 | 8-JAMB | 7'-0" |
| 10 | JMBTRM | 7'-6" |
| 11 | JMBSTF | 7'-6" |
| 12 | 8-HEAD | 10'-0" |
| 13 | HEDTRM | 11'-0" |
| 14 | BT2 | 31'-6" |
| 15 | HEDTRM | 7'-6" |

| | | |
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| CAMP PINEWOOD | | DRAWN BY SB |
| CAMP PINEWOOD | | DRAWING NUMBER of |

| FASTENER TABLE | | |
|----------------|------------|------------|
| ○ ID | SCREW PART | SPACE (in) |
| 1 | #12S | 12.0 |
| 2 | #12S | 6.0 |



PANEL ZONE LAYOUT
(Minimum Fastener Spacing)



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| CAMP PINEWOOD | | DRAWN BY SB |
| CAMP PINEWOOD | | DRAWING NUMBER OF |